## TRAFFIC ANALYSIS REPORT

FOR<br>\title{ RIVERSIDE ESTATES SUBDIVISION }

NW PACIFIC HIGHWAY

CITY OF LA CENTER

SUBMITTED BY

May 2017
Project 17-13

## TRAFFIC ANALYSIS REPORT

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## RIVERSIDE ESTATES SUBDIVISION

NW PACIFIC HIGHWAY

## CITY OF LA CENTER

## Prepared By

## CHARBONNEAU Engineering LLC



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## INTRODUCTION

This traffic study has been prepared to evaluate and document the operations and safety conditions for the Riverside Estates Subdivision development being planned in La Center, Washington. The development will build a 99 single-family homes and 306 apartment units. The project site is located in north La Center on the south side of Pacific Highway near the intersection of NW Larson Drive. Figure 'a' in the appendix is a vicinity map highlighting the project location.

In accordance with the City's requirements the study area was defined as the surrounding neighborhood including the site access point at Pacific Highway and several key intersections including NW Lacenter Road at Timmen Road and Pacific Highway at $5^{\text {th }}$ Street and at $10^{\text {th }}$ Street.

## TRAFFIC ANALYSIS CONSIDERATIONS

In the project scope established with City of La Center staff, a number of important elements were identified and considered in the study.

- Inventory and record pertinent information such as traffic control devices, circulation patterns, lane conditions, pedestrian \& bicycle facilities, transit zones, parking, and street characteristics.
- Record data on typical weekdays during the AM \& PM peak traffic hours.
- Obtain traffic counts for the intersections on Pacific Highway at Larson Drive, $10^{\text {th }}$ Street, and $5^{\text {th }}$ Street and on Lacenter Drive at Timmen Road.
- The project buildout is estimated to occur in year 2020. Three years of traffic growth at $2 \%$ per year was applied to establish the year 2020 background volumes. The City confirmed that in-process traffic for the Sunrise Terrace development was applicable.
- Prepare trip generation for 99 single-family homes and 306 apartments using the latest edition of the ITE Trip Generation manual (9 ${ }^{\text {th }}$ edition, year 2012).
- Level of service (LOS) analysis of the study intersections to measure the approach delays and LOS for comparison to City of La Center standards.
- Review intersection sight distance at the proposed access on Pacific Highway.
- Prepare peak hour signal warrant and left turn lane warrant.
- Review crash data furnished by WSDOT. Identify crash rates at the study intersections.
- Review the WSDOT Six Year Transportation Improvement Program from 2016 to $\underline{2021}$ to identify future projects covered in La Center.


## SITE DESCRIPTION, STREETS, ACCESS, AND CRITICAL INTERSECTIONS

Development of the Riverside Estates Subdivision project will include construction of 99 single-family homes and 306 apartment units. The project's location is situated on a 44.48 acre parcel (\#986028830) at the corner of Pacific Highway and Larson Drive. The address is

1514 NW $339^{\text {th }}$ Street, La Center. The property is currently vacant and is used for agriculture or pasture.

Access to the proposed development includes one approach on Pacific Highway. The project site plan (Figure 'b') illustrates the access location. The new approach will have stop control. The site's internal streets will include sidewalks and connectivity within for site circulation purposes.

The study intersections on Pacific Highway at $10^{\text {th }}$ Street and at $5^{\text {th }}$ Street are currently controlled by stop signs. The intersection at Lacenter Road and Timmen Road is controlled by stop signing. In the future according to WSDOT's Six Year TIP for 2016-2021 the intersection of Lacenter Road at Timmen Road will become signalized or converted to a roundabout. The existing and proposed lane configurations and traffic control are presented in Figure 'c'.

Pacific Highway adjacent to the site is classified as a major arterial and contains one travel lane in each direction. The travel speed is posted at 35 MPH. There are no bike lanes or sidewalks on the street. No on-street parking is permitted. Based on AASHTO the required intersection sight distance at the proposed access point is 390 feet.

Pacific Highway at $\mathbf{1 0}^{\text {th }}$ Street is configured as a tee-shaped intersection containing stop signing on the $10^{\text {th }}$ Street approach where there are separate left and right turn lanes. There are no separate turn lanes on Pacific Highway. The travel speed is posted at 25 MPH on Pacific Highway. There are no bike lanes. Sidewalks exist on both streets.

Pacific Highway at $5^{\text {th }}$ Street is four-way intersection with stop control on the $5^{\text {th }}$ Street approaches. There are no separate turn lanes at this location. The travel speed is posted at 25 MPH on Pacific Highway. There are no bike lanes. Sidewalks exist on both streets.

Lacenter Road at Timmen Road is configured as a tee-shaped intersection containing stop signing on the Timmen Road approach where there are separate left and right turn lanes. There is a separate westbound left turn lane on Pacific Highway. The travel speed is posted at 40 MPH on Pacific Highway. There are no bike lanes or sidewalks at this location.

## TRAFFIC OPERATIONAL ANALYSIS

In order to evaluate traffic flow and delay at the study intersections level of service (LOS) and safety conditions were determined. The intersections evaluated included Pacific Highway at the future site access, $10^{\text {th }}$ Street, and $5{ }^{\text {th }}$ Street and Lacenter Road at Timmen Road. The results included identification of the LOS and average delay per vehicle in the peak hours for the following scenarios:

- Year 2017 Existing Traffic
- Year 2020 Background Traffic
- Year 2020 Total Traffic

In order to perform the LOS analysis at the critical intersections video traffic counts were conducted during the AM peak (7:00-9:00AM) \& PM peak (4:00-6:00 PM) traffic hours. Figure 1 depicts the existing AM \& PM peak hour traffic volumes.

Three years of traffic growth ( $2 \%$ per year) plus in-process traffic has been added to the existing volumes to account for the background traffic volumes. The in-process traffic included the Sunrise Terrace development as referenced according to City staff and is shown on Figure 2. The year 2020 background traffic volumes are illustrated in Figure 3.

The year 2020 total traffic scenario (background plus site generated traffic) is presented in Figure 6.

## VEHICULAR TRIP GENERATION

Trip rates presented in the Institute of Transportation Engineers (ITE) Trip Generation manual $9^{\text {th }}$ edition (year 2012) were utilized to estimate the site's trip generation. The trip generation is summarized in Table 1.

Table 1 Trip Generation Summary

| ITE Land Use | Units <br> (\#) | Weekday |  |  |  |  |  |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | ADT | AM Peak Hour |  |  | PM Peak Hour |  |  |  |
|  |  |  | Total | Enter | Exit | Total | Enter | Exit |  |
| Single-Family (\#210) | 99 |  |  |  |  |  |  |  |  |
| Generation Rate $^{1}$ |  | 9.52 | 0.75 | $25 \%$ | $75 \%$ | 1.00 | $63 \%$ | $37 \%$ |  |
| Site Trips |  | $\mathbf{9 4 2}$ | $\mathbf{7 4}$ | 19 | 55 | $\mathbf{9 9}$ | 62 | 37 |  |
| Apartment (\#220) | 306 |  |  |  |  |  |  |  |  |
| Generation Rate $^{1}$ |  | 6.65 | 0.51 | $20 \%$ | $80 \%$ | 0.62 | $65 \%$ | $35 \%$ |  |
| Site Trips |  | $\mathbf{2 , 0 3 5}$ | $\mathbf{1 5 6}$ | 31 | 125 | $\mathbf{1 9 0}$ | 124 | 66 |  |
| Site Total Trips |  | $\mathbf{2 , 9 7 7}$ | $\mathbf{2 3 0}$ | $\mathbf{5 0}$ | $\mathbf{1 8 0}$ | $\mathbf{2 8 9}$ | $\mathbf{1 8 6}$ | $\mathbf{1 0 3}$ |  |

${ }^{1}$ Source: Trip Generation, 9th Edition, ITE, 2012, average rates.

The proposed development is expected to generate a net total 2,977 daily trips, 230 AM peak hour trips, and 289 PM peak hour trips.

The trip distribution was based on the existing traffic counts, intersection traffic control, site access location, and engineering judgment. Figure 4 presents the trip distribution results and Figure 5 displays the trip assignments.

## CAPACITY ANALYSIS

Capacity analyses were performed to determine the levels of service for the weekday peak hours. Synchro v9.1 software based on the year 2010 Highway Capacity Manual
methodology was used to determine the LOS and approach delays for the study intersections. The results are summarized in the following table. Copies of the capacity analysis summaries are included in the appendix.

Table 2 Capacity Analysis Summary


Notes: 2010 Highway Capacity Manual methodology used in analysis, Synchro v9.
Crit. Mov't - Critical movement or critical approach.
${ }^{1}$ Mitigation with Signal

According to the City's Comprehensive Plan policy the minimum acceptable level of service mobility standard for stop controlled intersections is LOS `E`. As documented in the Table 2 summary all of the study intersections except Pacific Highway at $5^{\text {th }}$ Street will operate at LOS `E` or better through the year 2020 total traffic scenario.

At Pacific Highway and $5^{\text {th }}$ Street the intersection will experience LOS `F` with 63.6 seconds of delay in the AM peak hour in the year 2020 total traffic scenario. The failing approach is the east leg with contains a heavy westbound to southbound left turn demand with 159 vehicles in the AM peak hour. Implementing a separate westbound left turn lane will not upgrade the intersection to acceptable LOS as the left turn movement still fails (LOS `F", delay 56 seconds). To mitigate this condition will require signalization with the service level improving to LOS `B` in the AM peak hour. However, signalization is not warranted and is not recommended. It is noted that the City will construct a roundabout at the intersection of $4^{\text {th }}$ Street and Pacific Highway in 2017 which is anticipated to relieve some of the traffic load on $5^{\text {th }}$ Street and improve the LOS condition.

Generally, LOS 'A', 'B', 'C', and ' D ' are desirable service levels ranging from no vehicle delays to average or longer than average delays in the peak hours. Level ' $E$ ' represents longer delays and is considered to be the limit of acceptable delay for unsignalized and signalized intersections. Signalization warrants need to be reviewed and signals considered only if warrants are met. Level ' $F$ ' indicates that intersection improvements, such as widening and signalization, may be required. According to the Highway Capacity Manual (HCM), the following delay times are associated with the LOS at stop controlled unsignalized and signalized intersections.

Level of Service criteria defined in Highway Capacity Manual

| Level of Service <br> (LOS) | Unsignalized Control <br> Stopped Delay (sec/veh) | Signalized Control <br> Stopped Delay (sec/veh) |
| :---: | :---: | :---: |
| A | $\leq 10$ | $\leq 10$ |
| B | $>10$ and $\leq 15$ | $>10$ and $\leq 20$ |
| C | $>15$ and $\leq 25$ | $>20$ and $\leq 35$ |
| D | $>25$ and $\leq 35$ | $>35$ and $\leq 55$ |
| E | $>35$ and $\leq 50$ | $>55$ and $\leq 80$ |
| F | $>50$ | $>80$ |

## QUEUING ANALYSIS

Queue length demand for the proposed site access on Pacific Highway was determined with the capacity analyses. The results based on the $95^{\text {th }}$ percentile queue rating indicated that for the year 2020 total traffic scenario queues on the stop approach in the AM \& PM peak hours will not exceed one to two vehicles. No westbound queuing on Pacific Highway at the site access is anticipated in the peak hours.

The LOS reports containing the queue results are contained in the appendix.

## SIGHT DISTANCE

Sight distance at the proposed access on Pacific Highway was reviewed in the field in accordance with the AASHTO standards. Using the posted travel speed of 35 MPH an intersection sight distance of 390 feet is required in both directions. The length of sight distance was determined to exceed 500 feet to the north and equated to 450 feet to the south. Therefore, the intersection sight distance standard will be met.

## LEFT TURN LANE REQUIREMENTS

Left turn lane needs were evaluated for the peak hour conditions at the site access point on Pacific Highway and on Pacific Highway at $10^{\text {th }}$ Street. Based on the warrant results a left turn lane is warranted on Pacific Highway (northbound) at the site access in the PM peak hour for year 2020 total traffic condition. Street improvements planned on Pacific Highway at the site's access include construction of a two-way left turn lane. A southbound left turn lane on Pacific Highway at $10^{\text {th }}$ Street is not warranted. The warrant curve results are included in the appendix.

## TRAFFIC SIGNAL WARRANTS

The peak hour signal warrant was evaluated for the stop controlled study intersections including the site access and Pacific Highway. The peak hour warrant data is included in the appendix.

The intersection of Lacenter Road at Timmen Road met the peak hour signal warrant in the PM peak hour for the existing, year 2020 background, and year 2020 total traffic scenarios. Therefore, the signal need is not attributed to the development and is not proposed in conjunction with the project. According to WSDOT's Six Year TIP for 2016-2021 a traffic signal or roundabout improvement is planned at this location.

## ACCIDENT HISTORY

Crash data for the study intersections on Pacific Highway and Lacenter Road was obtained from WSDOT staff and reviewed to identify potential safety issues. The latest available data covered the years 2012-2016.

The accident rates presented in Table 3 below are based on the number of accidents per million entering vehicles (MEV) per year. Typically, an intersection is not considered unsafe unless the crash rate exceeds the threshold value of 1.0 accidents per MEV.

Table 3 Crash Rate Summary

| Intersection | Accident <br> History <br> (Years) | Number of <br> Accidents | Accidents |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| per year |  |  |  | | PM |
| :---: |
| Peak Vol. |
| Entering |
| (veh/hr) | | Annual |
| :---: |
| Traffic <br> Entering <br> (veh/yr) |
| Accident <br> rate per <br> M.E.V. |
| Pacific Highway at Larson Drive |
| Pacific Highway at 10th Street |
| Pacific Highway at 5th Street |
| Lacenter Road at Timmen Road |

* M.E.V. - million entering vehicles.

None of the intersections experienced a crash rate above 0.12 crashes per MEV per year indicating safety mitigation is not necessary.

## PEDESTRIANS, BICYCLES, \& BUSES

Sidewalk will be provided along both sides of the streets constructed internally within site's property. Sidewalk will also be constructed along the site's frontage adjacent to Pacific Highway.

No bicycle lanes are provided on Pacific Highway along the development's frontage. New bike lanes are planned with the project along the site's property frontage.

C-Tran provides limited service to La Center with the Connector route which runs on weekdays only. The service operates between downtown La Center with a stop at the $4^{\text {th }}$ Street Park \& Ride to the $99^{\text {th }}$ Street Transit Center in Vancouver.

## SUMMARY AND RECOMMENDATIONS

The traffic study for Riverside Estates Subdivision has been prepared to determine the potential impacts at the site access point on Pacific Highway and several study intersections including Pacific Highway at $10^{\text {th }}$ Street and $5^{\text {th }}$ Street and Lacenter Road at Timmen Road. Development of the site includes 99 single-family homes and 306 apartment units. Trip generation is projected to be 2,977 daily trips with 230 AM peak hour trips and 289 PM peak hour trips.

Sight distance at the proposed access on Pacific Highway was reviewed in the field accordance with the AASHTO standards. Using the posted travel speed of 35 MPH an intersection sight distance of 390 feet is required in both directions. The length of sight distance was determined to exceed 500 feet to the north and equated to 450 feet to the south. Therefore, the intersection sight distance standard will be met. The sight distance standards shall be maintained for safety purposes and potential obstruction to the sightlines by vegetation, walls, parking, signing, buildings or other items must be avoided.

According to the City's Comprehensive Plan policy the minimum acceptable level of service mobility standard for stop controlled intersections is LOS `E`. All of the study intersections except Pacific Highway at $5^{\text {th }}$ Street will operate at LOS `E` or better through the year 2020 total traffic scenario. At Pacific Highway and $5^{\text {th }}$ Street the intersection will experience LOS `F' with 63.6 seconds of delay in the AM peak hour in the year 2020 total traffic scenario. The failing approach is the east leg with contains a heavy westbound to southbound left turn volume in the AM peak hour. Implementing a separate westbound left turn lane will not upgrade the intersection to acceptable LOS. To mitigate the condition will require signalization with the service level improving to LOS `B` in the AM peak hour. However, signalization is not warranted and is not recommended. It is noted that the City will construct
a roundabout this year at the intersection of $4^{\text {th }}$ Street and Pacific Highway which is anticipated to relieve some of the traffic load on $5^{\text {th }}$ Street and improve the LOS condition.

Queue length demand for the proposed site access on Pacific Highway was determined with the capacity analyses. The results based on the $95^{\text {th }}$ percentile queue rating indicated that for the year 2020 total traffic scenario queues on the stop approach in the AM \& PM peak hours will not exceed one to two vehicles. No westbound queuing on Pacific Highway at the site access is anticipated in the peak hours.

Crash data for the study intersections was obtained from WSDOT staff and reviewed to identify potential safety issues. The latest five years of available data was reviewed. None of the intersection crash rates exceed 0.12 accidents per MEV per year indicating safety mitigation is not necessary.

Based on evaluation of the study intersections including level of service conditions and vehicle delays, crash history, and warrants no intersection improvements beyond those planned at the site access and frontage are required in conjunction with the proposed development. The site access approach to Pacific Highway will require stop sign control and inclusion of a stop bar pavement marking.

## APPENDIX

- Vicinity Map
- Site Plan
- Lane Configurations and Traffic Control
- Traffic Flow Diagrams

Figure $1 \quad 2017$ Existing Traffic (AM \& PM)
Figure 2 In-Process Traffic
Figure 32020 Background Traffic
Figure 4 Trip Distribution
Figure 5 Trip Assignment
Figure $6 \quad 2020$ Total Traffic

- Traffic Count Data
- In-Process Traffic - Sunrise Terrace
- Left Turn Lane Warrant Worksheets
- Peak Hour Signal Warrant
- Crash History Summary (furnished by WSDOT)
- 2016-2021 Six Year TIP
- Synchro v9.1 Capacity Analysis Worksheets



## Riverside Estates Subdivision

Located in the SE $1 / 4$ of Section 33, T5N, R1E and









## Peak Hour Summary

## All Traffic Data

Clay Carney
(503) 833-2740

# NW Pacific Hwy \& NW Larson Dr 

7:45 AM to 8:45 AM
Tuesday, April 18, 2017


Bikes 0

| Approach | PHF | HV\% | Volume |
| :---: | :---: | :---: | :---: |
| EB | 0.42 | $0.0 \%$ | 5 |
| WB | 0.00 | $0.0 \%$ | 0 |
| NB | 0.75 | $3.6 \%$ | 84 |
| SB | 0.79 | $6.1 \%$ | 114 |
| Intersection | 0.98 | $4.9 \%$ | 203 |

## Peak Hour Summary

## All Traffic Data

Clay Carney
(503) 833-2740

# NW Pacific Hwy \& NW Larson Dr 

4:15 PM to 5:15 PM
Tuesday, April 18, 2017


| Approach | PHF | HV\% | Volume |
| :---: | :---: | :---: | :---: |
| EB | 0.50 | $0.0 \%$ | 2 |
| WB | 0.00 | $0.0 \%$ | 0 |
| NB | 0.95 | $0.7 \%$ | 140 |
| SB | 0.83 | $1.0 \%$ | 96 |
| Intersection | 0.90 | $0.8 \%$ | 238 |






## Peak Hour Summary

```
All Traffic Data
```

Clay Carney (503) 833-2740

# NW Timmen Rd \& NW Lacenter Rd 

## 7:15 AM to 8:15 AM

Tuesday, April 18, 2017


| Approach | PHF | HV\% | Volume |
| :---: | :---: | :---: | :---: |
| EB | 0.68 | $8.8 \%$ | 227 |
| WB | 0.89 | $4.4 \%$ | 616 |
| NB | 0.63 | $8.8 \%$ | 80 |
| SB | 0.00 | $0.0 \%$ | 0 |
| Intersection | 0.93 | $5.9 \%$ | 923 |

## Peak Hour Summary

```
All Traffic Data
```

Clay Carney (503) 833-2740

NW Timmen Rd \& NW Lacenter Rd
4:45 PM to 5:45 PM
Tuesday, April 18, 2017


| Approach | PHF | HV\% | Volume |
| :---: | :---: | :---: | :---: |
| EB | 0.87 | $0.6 \%$ | 648 |
| WB | 0.87 | $1.4 \%$ | 367 |
| NB | 0.81 | $4.2 \%$ | 166 |
| SB | 0.00 | $0.0 \%$ | 0 |
| Intersection | 0.96 | $1.4 \%$ | 1,181 |




Left Turn Lane Criterion for 2020 Total Traffic

| Intersection | Mov't | Analysis Period | Speed (mph) | \%Total DHV <br> Turning Left | Total DHV | Left Turn Lane Required |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Pacific Highway at Site Access (NW 15 ${ }^{\text {th }}$ Street) | NB | AM Peak Hour | 35 | 15.2\% | 296 | No |
|  |  | PM Peak Hour |  | 37.7\% | 448 | Yes |
| Pacific Highway at 10th Street | SB | AM Peak Hour | 25 | 2.4\% | 453 | No |
|  |  | PM Peak Hour |  | 4.3\% | 585 | No |
| Pacific Highway at 5th Street | NB | AM Peak Hour | 25 | 0.37\% | 542 | No |
|  |  | PM Peak Hour |  | 1.8\% | 669 | No |

Total Traffic Volume (DHV)
Source: Oregon Department of Transportation Analysis Procedure Manual

Engineering LLC

Warrant 3: Peak Hour
17-12 Riverside Estates
1: Pacific Highway at Site Access (NE15th Street)
Intersection Information

|  | Major Street | Minor Street |
| :--- | :--- | :--- |
| Street Name | Pacific Highway | Site Access/NW 15th Street |
| Direction | EB/WB/NWB | NB |
| Number of Lanes | 1 | 1 |
| Approch Speed | 35 | 25 |

## Warrant 3 Met? No

## Details

| Low Population: | Yes |  |  |
| :--- | :--- | :--- | :--- |
| Condition A Met | No |  |  |
| Notes | 0 Hours met (1 required) | Condition B Met | No |
| Minor Approach Time Delay Condition Met? | Notes | 0 Hours met (1 required) |  |
| Minor Approach Volume Condition Met? | Not Met |  |  |
| Total Entering Intersection Volume Condition Met? | Met |  |  |

## Peak Hour Vehicular Volume

Community Population Less Than 10,000 or Major Street Approach Speed Above 40 mph


Warrant 3: Peak Hour
1: Pacific Highway at Site Access (NE15th Street)

| Hour | Major Street <br> Total All <br> Approaches (vph) | Minor Street <br> Highest Volume <br> Approach (vph) |
| :---: | :---: | :---: |
| $7: 45$ | 269 | 185 |
| $16: 15$ | 448 | 105 |

Warrant 3: Peak Hour
2: Pacific Highway at W 10th Street
Intersection Information

|  | Major Street | Minor Street |
| :--- | :--- | :--- |
| Street Name | Pacific Highway | W 10th Street |
| Direction | EB/WB | SB |
| Number of Lanes | 1 | 2 |
| Approch Speed | 25 | 25 |

## Warrant 3 Met? <br> No

Details

| Low Population: | Yes |  |  |
| :--- | :--- | :--- | :--- |
| Condition A Met | No |  |  |
| Notes | 0 Hours met (1 required) | Condition B Met | No |
| Minor Approach Time Delay Condition Met? | Notes | 0 Hours met (1 required) |  |
| Minor Approach Volume Condition Met? | Not Met |  |  |
| Total Entering Intersection Volume Condition Met? | Met |  |  |

## Peak Hour Vehicular Volume

Community Population Less Than 10,000 or Major Street Approach Speed Above 40 mph


Warrant 3: Peak Hour
2: Pacific Highway at W 10th Street

| Hour | Major Street <br> Total All <br> Approaches (vph) | Minor Street <br> Highest Volume <br> Approach (vph) |
| :---: | :---: | :---: |
| $7: 30$ | 453 | 92 |
| $16: 00$ | 585 | 69 |

Warrant 3: Peak Hour

## 3: Pacific Highway at W 5th Street

Intersection Information

|  | Major Street | Minor Street |
| :--- | :--- | :--- |
| Street Name | Pacific Highway | W 5th Street |
| Direction | NB/SB | EB/WB |
| Number of Lanes | 1 | 1 |
| Approch Speed | 25 | 25 |

## Warrant 3 Met? <br> No

Details

| Low Population: | Yes |  |  |
| :--- | :--- | :--- | :--- |
| Condition A Met | No |  |  |
| Notes | 0 Hours met (1 required) | Condition B Met | No |
| Minor Approach Time Delay Condition Met? | Notes | 0 Hours met (1 required) |  |
| Minor Approach Volume Condition Met? | Not Met |  |  |
| Total Entering Intersection Volume Condition Met? | Met |  |  |

Peak Hour Vehicular Volume
Community Population Less Than 10,000 or Major Street Approach Speed Above 40 mph


Warrant 3: Peak Hour
3: Pacific Highway at W 5th Street

| Hour | Major Street <br> Total All <br> Approaches (vph) | Minor Street <br> Highest Volume <br> Approach (vph) |
| :---: | :---: | :---: |
| $7: 30$ | 542 | 174 |
| $17: 00$ | 669 | 79 |

Warrant 3: Peak Hour
4: Lacenter Road at Timmen Road
Intersection Information

|  | Major Street | Minor Street |
| :--- | :--- | :--- |
| Street Name | Lacenter | Timmen |
| Direction | EB/WB | NB |
| Number of Lanes | 2 | 2 |
| Approch Speed | 40 | 40 |

## Warrant 3 Met? Yes

Details

| Low Population: | Yes |  |  |
| :--- | :--- | :--- | :--- |
| Condition A Met | No |  |  |
| Notes | 0 Hours met (1 required) | Condition B Met | Yes |
| Minor Approach Time Delay Condition Met? | Notes | 1 Hours met (1 required) |  |
| Minor Approach Volume Condition Met? | Not Met |  |  |
| Total Entering Intersection Volume Condition Met? | Not Met |  |  |

## Peak Hour Vehicular Volume

Community Population Less Than 10,000 or Major Street Approach Speed Above 40 mph


Warrant 3: Peak Hour
4: Lacenter Road at Timmen Road

| Hour | Major Street <br> Total All <br> Approaches (vph) | Minor Street <br> Highest Volume <br> Approach (vph) |
| :---: | :---: | :---: |
| $7: 15$ | 1,123 | 95 |
| $16: 45$ | 1,343 | 215 |

OFFICER REPORTED CRASHES THAT OCCURREDat OR in the vicinity of THE FOLLOWING INTERSECTIONS IN THE CITY OF LA CENTER
PACIFIC HWY @ LARSON DR
PACIFIC HWY @ 10th ST
PACIFIC HWY @ 5th ST
LA CENTER RD @ TIMMEN RD
01/01/2012-12/31/2016
Under 23 U.S. Code $\$ 409$ and 23 U.S. Code 148 , safety data, reports, surveys, schedules, lists compiled or collected for the purpose of identifising, evaluatitng, or
planning the safery enhancement of potential crash sites, hazardous roadway conditions, or railway-highway crossings are not subbiect to discovery or admitted into
planning the saffety enhancement of potential crash sites, hazardous roadway conditions, or railway-highway crossings are not subject to discovery or admitte into
evidence in a Federal or State court proceeding or considered for other purposes in any action for damages arising from any occurrence at a location mentioned o


| JUNCTION RELATIONSHIP | ROADWAY SURFACE CONDITION | LIGHTING CONDITION | FIRST COLLISION TYPE / OBJECT STRUCK | VEHICLE 1 ACTION | VEHICLE 2 ACTION | VEHICLE 1 COMPASS DIRECTION FROM | VEHICLE 1 COMPASS DIRECTION TO |  |  | mV DRIVER CONTRIBUTING CIRCUMSTANCE 1 (UNIT 1) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Not at Intersection and Not Related | Dry | Dark-No Street Lights | Roadway Ditch | Going Straight Ahead |  | West | East |  |  | Exceeding Stated Speed Limit |
| Not at Intersection and Not Related | Wet | Daylight | From same direction - both going straight - both moving - sideswipe | Starting in Traffic Lane | Overtaking and Passing | Northwest | Southeast | Northwest | Southeast | Other |
| At Driveway | Dry | Daylight | Vehicle going straight hits pedestrian | Going Straight Ahead |  | East | West |  |  | None |
| At Intersection and Related | Dry | Dark-No Street Lights | Entering at angle | Making Left Turn | Going Straight Ahead | Southeast | Southwest | Southwest | Northeast | Did Not Grant RW to Vehicle |
| Not at Intersection and Not Related | Dry | Daylight | From same direction - both going straight - both moving - rear-end | Slowing | Going Straight Ahead | South | North | South | North | None |
| Not at Intersection and Not Related | Dry | Dawn | Tree or Stump (stationary) | Going Straight Ahead |  | Northeast | Southwest |  |  | Apparently III |
| Not at Intersection and Not Related | Ice | Daylight | Roadway Ditch | Going Straight Ahead |  | Northeast | Southwest |  |  | None |
| Not at Intersection and Not Related | Ice | Dark-No Street Lights | Roadway Ditch | Going Straight Ahead |  | East | West |  |  | None |
| Not at Intersection and Not Related | Ice | Dark-No Street Lights | Guardrail - Face | Going Straight Ahead |  | East | West |  |  | None |
| Not at Intersection and Not Related | Dry | Dark-No Street Lights | Vehicle Strikes Deer | Going Straight Ahead | Going Straight Ahead | South | North | North | South | Driver Not Distracted |
| At Driveway | We | Other | From same direction - one left turn - one straight | Overtaking and Passing | Making Left Turn | West | East | West | North | Driver Not Distracted |
| Not at Intersection and Not Related | Dry | Daylight | From same direction - both going straight - one stopped - rear-end | Stopped in Roadway | Going Straight Ahead | Vehicle Stopped | Vehicle Stopped | West | East | None |
| Not at Intersection and Not Related | Dry | Daylight | From same direction - both going straight - both moving - rear-end | Going Straight Ahead | Going Straight Ahead | East | West | East | We | Inattention |


| mV DRIVER CONTRIBUTING CIRCUMSTANCE 2 (UNIT 1) | MV DRIVER CONTRIBUTING CIRCUMSTANCE 3 (UNIT 1) | MV DRIVER CONTRIBUTING CIRCUMSTANCE 1 (UNIT 2) | MV DRIVER CONTRIBUTING CIRCUMSTANCE 2 (UNIT 2) | MV DRIVER CONTRIBUTING CIRCUMSTANCE 3 (UNIT 2) | PEDESTRIAN CONTRIBUTING CIRCUMSTANCE 1 (UNIT 2) | PEDESTRIAN CONTRIBUTING CIRCUMSTANCE 2 (UNIT 2) | PEDESTRIAN CONTRIBUTING CIRCUMSTANCE 3 (UNIT 2) | FIRST IMPACT LOCATION (City, County \& Misc Trafficways - 2010 forward) | WA STATE <br> PLANE <br> SOUTH - X <br> 2010 - <br> FORWARD | $\begin{array}{\|c\|} \hline \text { WA STATE } \\ \text { PLANE } \\ \text { SOUTH - Y } \\ 2010- \\ \text { FORWARD } \\ \hline \end{array}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  | Past the Outside Shoulder of Primary Trafficway | 1083833.899 | 202953.37 |
|  |  | Improper Passing |  |  |  |  |  | Lane of Primary Trafficway | 1086414.25 | 201833.04 |
|  |  |  |  |  | Did Not Grant RW to Vehicle |  |  | Lane of Primary Trafficway | 1086963.8 | 200705.73 |
|  |  | None |  |  |  |  |  | Lane of Primary Trafficway | 1085773.83 | 197292.799 |
| Follow Too Closely |  | Inattention |  |  |  |  |  | Lane of Primary Trafficway | 1085773.83 | 197292.79 |
|  |  |  |  |  |  |  |  | Past the Outside Shoulder of Primary Trafficway | 1085343.159 | 197039.209 |
|  |  |  |  |  |  |  |  | Past the Outside Shoulder of Primary Trafficway | 1084559.09 | 196810.85 |
|  |  |  |  |  |  |  |  | Past the Outside Shoulder of Primary Trafficway | 1086654.32 | 198264.959 |
|  |  |  |  |  |  |  |  | Outside Shoulder of Primary Trafficway | 1085567.129 | 197154.78 |
|  |  | Driver Not Distracted |  |  |  |  |  | Lane of Primary Trafficway | 1086181.72 | 197648.66 |
|  |  | None | Improper Passing |  |  |  |  | Lane of Primary Trafficway | 1085823.439 | 197340.429 |
|  |  | Inattention |  |  |  |  |  | Lane of Primary Trafficway | 1084880.73 | 196883.989 |
|  |  | None |  |  |  |  |  | Lane of Primary Trafficway | 1086589.33 | 195846.95 |

Six Year Transportation Improvement Program
From 2016 to 2021



| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 2.9 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | ${ }^{7}$ | 「 | F |  |  | $\uparrow$ |
| Traffic Vol, veh/h | 61 | 25 | 58 | 11 | 10 | 143 |
| Future Vol, veh/h | 61 | 25 | 58 | 11 | 10 | 143 |
| Conflicting Peds, \#/hr | 4 | 1 | 0 | 3 | 1 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 50 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 86 | 86 | 81 | 81 | 75 | 75 |
| Heavy Vehicles, \% | 2 | 2 | 3 | 3 | 6 | 6 |
| Mvmt Flow | 71 | 29 | 72 | 14 | 13 | 191 |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | * |  |  | * |  |  | \$ |  |  | \& |  |
| Traffic Vol, veh/h | 1 | 0 | 7 | 150 | 2 | 12 | 2 | 63 | 30 | 7 | 209 | 1 |
| Future Vol, veh/h | 1 | 0 | 7 | 150 | 2 | 12 | 2 | 63 | 30 | 7 | 209 | 1 |
| Conflicting Peds, \#/hr | 1 | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 2 |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |
| Veh in Median Storage, \# | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 50 | 50 | 50 | 54 | 54 | 54 | 74 | 74 | 74 | 79 | 79 | 79 |
| Heavy Vehicles, \% | 0 | 0 | 0 | 4 | 4 | 4 | 4 | 4 | 4 | 3 | 3 | 3 |
| Mvmt Flow | 2 | 0 | 14 | 278 | 4 | 22 | 3 | 85 | 41 | 9 | 265 | 1 |



HCM 2010 TWSC
12: Timmen Rd \& Lacenter Rd

| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh |  |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | F |  | ${ }^{1}$ | 4 | ${ }^{1}$ | 「 |
| Traffic Vol, veh/h | 225 | 2 | 113 | 503 | 11 | 69 |
| Future Vol, veh/h | 225 | 2 | 113 | 503 | 11 | 69 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | 100 | - | 100 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 245 | 2 | 123 | 547 | 12 | 75 |



| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh |  |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | ${ }^{*}$ | 「 | $\uparrow$ |  |  | * |
| Traffic Vol, veh/h | 17 | 48 | 130 | 47 | 24 | 94 |
| Future Vol, veh/h | 17 | 48 | 130 | 47 | 24 | 94 |
| Conflicting Peds, \#/hr | 2 | 2 | 0 | 2 | 2 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | S | None | - | None | - | None |
| Storage Length | 50 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 86 | 86 | 87 | 87 | 89 | 89 |
| Heavy Vehicles, \% | 2 | 2 | 3 | 3 | 6 | 6 |
| Mvmt Flow | 20 | 56 | 149 | 54 | 27 | 106 |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | * |  |  | $\$$ |  |  | * |  |  | * |  |
| Traffic Vol, veh/h | 1 | 1 | 1 | 63 | 0 | 11 | 11 | 193 | 18 | 10 | 139 | 2 |
| Future Vol, veh/h | 1 | 1 | 1 | 63 | 0 | 11 | 11 | 193 | 18 | 10 | 139 | 2 |
| Conflicting Peds, \#/hr | 1 | 0 | 1 | 5 | 0 | 5 | 1 | 0 | 5 | 5 | 0 | 1 |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |
| Veh in Median Storage, \# | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 38 | 38 | 38 | 93 | 93 | 93 | 72 | 72 | 72 | 88 | 88 | 88 |
| Heavy Vehicles, \% | 0 | 0 | 0 | 3 | 3 | 3 | 1 | 1 | 1 | 1 | 1 | 1 |
| Mvmt Flow | 3 | 3 | 3 | 68 | 0 | 12 | 15 | 268 | 25 | 11 | 158 | 2 |



HCM 2010 TWSC
12: Timmen Rd \& Lacenter Rd

| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh |  |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | 个 |  | ${ }^{1}$ | 4 | ${ }^{1}$ | 「 |
| Traffic Vol, veh/h | 633 | 15 | 71 | 296 | 12 | 154 |
| Future Vol, veh/h | 633 | 15 | 71 | 296 | 12 | 154 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | 100 | - | 100 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 87 | 87 | 87 | 87 | 81 | 81 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 728 | 17 | 82 | 340 | 15 | 190 |



| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh |  |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | ${ }^{*}$ | 「 | $\uparrow$ |  |  | $\uparrow$ |
| Traffic Vol, veh/h | 65 | 27 | 64 | 12 | 11 | 159 |
| Future Vol, veh/h | 65 | 27 | 64 | 12 | 11 | 159 |
| Conflicting Peds, \#/hr | 4 | 1 | 0 | 3 | 1 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 50 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 86 | 86 | 81 | 81 | 75 | 75 |
| Heavy Vehicles, \% | 2 | 2 | 3 | 3 | 6 | 6 |
| Mvmt Flow | 76 | 31 | 79 | 15 | 15 | 212 |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | * |  |  | * |  |  | \$ |  |  | \& |  |
| Traffic Vol, veh/h | 1 | 0 | 7 | 159 | 2 | 13 | 2 | 69 | 32 | 7 | 224 | 1 |
| Future Vol, veh/h | 1 | 0 | 7 | 159 | 2 | 13 | 2 | 69 | 32 | 7 | 224 | 1 |
| Conflicting Peds, \#/hr | 1 | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 2 |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |
| Veh in Median Storage, \# | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 50 | 50 | 50 | 54 | 54 | 54 | 74 | 74 | 74 | 79 | 79 | 79 |
| Heavy Vehicles, \% | 0 | 0 | 0 | 4 | 4 | 4 | 4 | 4 | 4 | 3 | 3 | 3 |
| Mvmt Flow | 2 | 0 | 14 | 294 | 4 | 24 | 3 | 93 | 43 | 9 | 284 | 1 |



HCM 2010 TWSC
12: Timmen Rd \& Lacenter Rd

| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh |  |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | F |  | ${ }^{1}$ | 4 | ${ }^{1}$ | 「 |
| Traffic Vol, veh/h | 253 | 2 | 130 | 573 | 12 | 76 |
| Future Vol, veh/h | 253 | 2 | 130 | 573 | 12 | 76 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | 100 | - | 100 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 275 | 2 | 141 | 623 | 13 | 83 |



| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh |  |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | ${ }^{*}$ | 「 | $\uparrow$ |  |  | * |
| Traffic Vol, veh/h | 18 | 51 | 142 | 50 | 25 | 108 |
| Future Vol, veh/h | 18 | 51 | 142 | 50 | 25 | 108 |
| Conflicting Peds, \#/hr | 4 | 1 | 0 | 3 | 1 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 50 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 86 | 86 | 81 | 81 | 75 | 75 |
| Heavy Vehicles, \% | 2 | 2 | 3 | 3 | 6 | 6 |
| Mvmt Flow | 21 | 59 | 175 | 62 | 33 | 144 |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh 4 |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | \& |  |  | \& |  |  | \& |  |  | \& |  |
| Traffic Vol, veh/h | 1 | 1 | 1 | 67 | 0 | 12 | 12 | 209 | 19 | 11 | 156 | 2 |
| Future Vol, veh/h | 1 | 1 | 1 | 67 | 0 | 12 | 12 | 209 | 19 | 11 | 156 | 2 |
| Conflicting Peds, \#/hr | 1 | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 2 |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |
| Veh in Median Storage, \# | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 |  |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 50 | 50 | 50 | 54 | 54 | 54 | 74 | 74 | 74 | 79 | 79 | 79 |
| Heavy Vehicles, \% | 0 | 0 | 0 | 4 | 4 | 4 | 4 | 4 | 4 | 3 | 3 | 3 |
| Mvmt Flow | 2 | 2 | 2 | 124 | 0 | 22 | 16 | 282 | 26 | 14 | 197 | 3 |



HCM 2010 TWSC
12: Timmen Rd \& Lacenter Rd

| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh |  |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | 个 |  | ${ }^{1}$ | 4 | ${ }^{1}$ | 「 |
| Traffic Vol, veh/h | 717 | 16 | 82 | 340 | 13 | 174 |
| Future Vol, veh/h | 717 | 16 | 82 | 340 | 13 | 174 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | 100 | - | 100 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 779 | 17 | 89 | 370 | 14 | 189 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 5.2 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | F |  |  | -1 |  |  |
| Traffic Vol, veh/h | 123 | 5 | 45 | 76 | 18 | 167 |
| Future Vol, veh/h | 123 | 5 | 45 | 76 | 18 | 167 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 134 | 5 | 49 | 83 | 20 | 182 |



| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 2.2 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | \% | 「 | $\uparrow$ |  |  | $\uparrow$ |
| Traffic Vol, veh/h | 65 | 27 | 109 | 12 | 11 | 321 |
| Future Vol, veh/h | 65 | 27 | 109 | 12 | 11 | 321 |
| Conflicting Peds, \#/hr | 4 | 1 | 0 | 3 | 1 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | S | None | - | None | - | None |
| Storage Length | 50 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 86 | 86 | 81 | 81 | 75 | 75 |
| Heavy Vehicles, \% | 2 | 2 | 3 | 3 | 6 | 6 |
| Mvmt Flow | 76 | 31 | 135 | 15 | 15 | 428 |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | * |  |  | * |  |  | \$ |  |  | \& |  |
| Traffic Vol, veh/h | 1 | 0 | 7 | 159 | 2 | 13 | 2 | 114 | 32 | 7 | 386 | 1 |
| Future Vol, veh/h | 1 | 0 | 7 | 159 | 2 | 13 | 2 | 114 | 32 | 7 | 386 | 1 |
| Conflicting Peds, \#/hr | 1 | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 2 |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |
| Veh in Median Storage, \# | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 50 | 50 | 50 | 54 | 54 | 54 | 74 | 74 | 74 | 79 | 79 | 79 |
| Heavy Vehicles, \% | 0 | 0 | 0 | 4 | 4 | 4 | 4 | 4 | 4 | 3 | 3 | 3 |
| Mvmt Flow | 2 | 0 | 14 | 294 | 4 | 24 | 3 | 154 | 43 | 9 | 489 | 1 |



HCM 2010 TWSC
12: Timmen Rd \& Lacenter Rd

| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh |  |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | 个 |  | ${ }^{1}$ | 4 | ${ }^{1}$ | 「 |
| Traffic Vol, veh/h | 283 | 2 | 157 | 681 | 12 | 83 |
| Future Vol, veh/h | 283 | 2 | 157 | 681 | 12 | 83 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | 100 | - | 100 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 308 | 2 | 171 | 740 | 13 | 90 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 4.4 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | 1 |  |  | $\uparrow$ |  |  |
| Traffic Vol, veh/h | 110 | 19 | 169 | 150 | 10 | 95 |
| Future Vol, veh/h | 110 | 19 | 169 | 150 | 10 | 95 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 120 | 21 | 184 | 163 | 11 | 103 |



| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh |  |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | ${ }^{7}$ | 「 | F |  |  | $\uparrow$ |
| Traffic Vol, veh/h | 18 | 51 | 309 | 50 | 25 | 201 |
| Future Vol, veh/h | 18 | 51 | 309 | 50 | 25 | 201 |
| Conflicting Peds, \#/hr | 4 | 1 | 0 | 3 | 1 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | S | None | - | None | - | None |
| Storage Length | 50 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 86 | 86 | 81 | 81 | 75 | 75 |
| Heavy Vehicles, \% | 2 | 2 | 3 | 3 | 6 | 6 |
| Mvmt Flow | 21 | 59 | 381 | 62 | 33 | 268 |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | * |  |  | * |  |  | * |  |  | \& |  |
| Traffic Vol, veh/h | 1 | 1 | 1 | 67 | 0 | 12 | 12 | 376 | 19 | 11 | 249 | 2 |
| Future Vol, veh/h | 1 | 1 | 1 | 67 | 0 | 12 | 12 | 376 | 19 | 11 | 249 | 2 |
| Conflicting Peds, \#/hr | 1 | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 2 |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |
| Veh in Median Storage, \# | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 50 | 50 | 50 | 54 | 54 | 54 | 74 | 74 | 74 | 79 | 79 | 79 |
| Heavy Vehicles, \% | 0 | 0 | 0 | 4 | 4 | 4 | 4 | 4 | 4 | 3 | 3 | 3 |
| Mvmt Flow | 2 | 2 | 2 | 124 | 0 | 22 | 16 | 508 | 26 | 14 | 315 | 3 |



HCM 2010 TWSC
12: Timmen Rd \& Lacenter Rd

| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh |  |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | t |  | ${ }^{1}$ | 4 | ${ }^{1}$ | 「 |
| Traffic Vol, veh/h | 828 | 16 | 97 | 402 | 13 | 202 |
| Future Vol, veh/h | 828 | 16 | 97 | 402 | 13 | 202 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | 100 | - | 100 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 900 | 17 | 105 | 437 | 14 | 220 |




Intersection Capacity Utilization 48.3\% ICU Level of Service A Analysis Period (min) 15

Splits and Phases: 7: Pacific Hwy \& 5th St


|  | 4 | $\rightarrow$ |  | 4 |  |  | $4$ | $\dagger$ | $p$ |  |  | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | * |  |  | \$ |  |  | \& |  |  | \& |  |
| Traffic Volume (vph) | 1 | 1 | 1 | 67 | 0 | 12 | 12 | 376 | 19 | 11 | 249 | 2 |
| Future Volume (vph) | 1 | 1 | 1 | 67 | 0 | 12 | 12 | 376 | 19 | 11 | 249 | 2 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Satd. Flow (prot) | 0 | 1773 | 0 | 0 | 1717 | 0 | 0 | 1814 | 0 | 0 | 1839 | 0 |
| Flt Permitted |  | 0.884 |  |  | 0.754 |  |  | 0.988 |  |  | 0.976 |  |
| Satd. Flow (perm) | 0 | 1592 | 0 | 0 | 1350 | 0 | 0 | 1794 | 0 | 0 | 1798 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  | 2 |  |  | 33 |  |  | 7 |  |  | 1 |  |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 30 |  |  | 30 |  |
| Link Distance (ft) |  | 343 |  |  | 440 |  |  | 420 |  |  | 345 |  |
| Travel Time (s) |  | 7.8 |  |  | 10.0 |  |  | 9.5 |  |  | 7.8 |  |
| Confl. Peds. (\#/hr) | 1 |  | 1 |  |  |  | 1 |  |  | 1 |  | 2 |
| Peak Hour Factor | 0.50 | 0.50 | 0.50 | 0.54 | 0.54 | 0.54 | 0.74 | 0.74 | 0.74 | 0.79 | 0.79 | 0.79 |
| Heavy Vehicles (\%) | 0\% | 0\% | 0\% | 4\% | 4\% | 4\% | 4\% | 4\% | 4\% | 3\% | 3\% | 3\% |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 6 | 0 | 0 | 146 | 0 | 0 | 550 | 0 | 0 | 332 | 0 |
| Turn Type | Perm | NA |  | Perm | NA |  | Perm | NA |  | Perm | NA |  |
| Protected Phases |  | 4 |  |  | 8 |  |  | 2 |  |  | 6 |  |
| Permitted Phases | 4 |  |  | 8 |  |  | 2 |  |  | 6 |  |  |
| Total Split (s) | 22.5 | 22.5 |  | 22.5 | 22.5 |  | 27.5 | 27.5 |  | 27.5 | 27.5 |  |
| Total Lost Time (s) |  | 4.5 |  |  | 4.5 |  |  | 4.5 |  |  | 4.5 |  |
| Act Effct Green (s) |  | 8.9 |  |  | 8.9 |  |  | 24.1 |  |  | 24.1 |  |
| Actuated g/C Ratio |  | 0.23 |  |  | 0.23 |  |  | 0.63 |  |  | 0.63 |  |
| v/c Ratio |  | 0.02 |  |  | 0.43 |  |  | 0.48 |  |  | 0.29 |  |
| Control Delay |  | 10.0 |  |  | 14.7 |  |  | 8.1 |  |  | 6.3 |  |
| Queue Delay |  | 0.0 |  |  | 0.0 |  |  | 0.0 |  |  | 0.0 |  |
| Total Delay |  | 10.0 |  |  | 14.7 |  |  | 8.1 |  |  | 6.3 |  |
| LOS |  | A |  |  | B |  |  | A |  |  | A |  |
| Approach Delay |  | 10.0 |  |  | 14.7 |  |  | 8.1 |  |  | 6.3 |  |
| Approach LOS |  | A |  |  | B |  |  | A |  |  | A |  |
| Queue Length 50th (ft) |  | 1 |  |  | 19 |  |  | 64 |  |  | 33 |  |
| Queue Length 95th (ft) |  | 4 |  |  | 29 |  |  | 116 |  |  | 72 |  |
| Internal Link Dist (ft) |  | 263 |  |  | 360 |  |  | 340 |  |  | 265 |  |
| Turn Bay Length (ft) |  |  |  |  |  |  |  |  |  |  |  |  |
| Base Capacity (vph) |  | 777 |  |  | 674 |  |  | 1148 |  |  | 1149 |  |
| Starvation Cap Reductn |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Spillback Cap Reductn |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Storage Cap Reductn |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Reduced v/c Ratio |  | 0.01 |  |  | 0.22 |  |  | 0.48 |  |  | 0.29 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Area Type: Other |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 50 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length: 38.2 |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type: Actuated-Uncoordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v/c Ratio: 0.48 |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay: 8.5 |  |  |  | Intersection LOS: A |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 42.7\% |  |  |  | ICU Level of Service A |  |  |  |  |  |  |  |  |

Analysis Period (min) 15
Splits and Phases: 7: Pacific Hwy \& 5th St


