TRANSPORTATION IMPACT STUDY

FOR

ASA'S VIEW SUBDIVISION

2313 NE LOCKWOOD CREEK ROAD

CITY OF LA CENTER, WASHINGTON



PREPARED BY
KELLY ENGINEERING

February 2022

TRANSPORTATION IMPACT STUDY

Asa's View Subdivision

City of La Center, Washington

February 21, 2022

Prepared for:

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INTRODUCTION

A transportation impact study (TIS) for the Asa's View Subdivision was conducted to determine the potential traffic related impacts of the development to the surrounding roadway system. The development will consist of 69 single family detached homes. The site is located at 2313 NE Lockwood Creek Road (Tax Lots 39 and 102 of Section 2, T4N R1E WM) in La Center, Washington. The zoning designation for the 16.56 acre site is LDR-7.5.

The site consists of one home that is served by a private road/driveway. The home will be demolished. Land uses within the vicinity of the site consist of undeveloped land and single family homes. The La Center Middle School is adjacent to the site and to the west. The La Center Elementary School, Holley Park, La Center Community Library and a post office are located within one mile of the site on NE Lockwood Creek Road. A vicinity map, aerial photograph and proposed development plan are shown in Figures 1a, 1b and 1c.

Roadway Characteristics

The site will have access onto NE Lockwood Creek Road through a public street that will align with NE 24th Avenue to the north. NE Lockwood Creek Road along the site frontage is a two lane paved roadway with no shoulders. Double yellow striping is along the centerline of the roadway indicating that passing is prohibited. The posted speed limit is 25 mph. The roadway is classified as a Minor Arterial.

Half street improvements will be constructed along the site frontage of NE Lockwood Creek Road for a plat per the Minor Arterial "A" Standard Detail. The streets within the development will be either a Neighborhood or Local Street Standard per the Engineering Standards depending on the average daily traffic (ADT) of the roadways.

The study area intersections in this report are controlled by stop signs on the minor street approaches and a roundabout at the Pacific Highway/W 4th Street intersection. The lane configurations for the intersections are shown in Figure 2.

Traffic Volumes

The traffic counts in this report were conducted from 7:00 to 9:00 am and 4:00 to 6:00 pm during September 2018, July 2021 and February 2022. The traffic counts were conducted to determine the peak hours. The peak hour at an intersection is the one hour time period when traffic on the adjacent streets are the highest and congestion is most likely to occur. The traffic counts conducted during September 2018 were factored by a rate of 3.9% per year to current year 2022 volumes as based on annual population growth. The existing traffic volumes are shown in Figures 3a and 3b. The adjusted existing traffic volumes are shown in Figures 4a and 4b. The raw traffic count data is included in Appendix A.

Trip Generation/Distribution

The Asa's View Subdivision will generate approximately 642 new trips per day. A trip is a one directional vehicle movement. 50 trips will occur during the weekday AM peak hour and 67 trips will occur during the PM peak hour, ITE <u>Trip Generation Manual</u>, 10th edition. Credits were given for the existing home to be demolished. The trip generation rates are shown in Table 1.

Table 1
Site Traffic Generation
Asa's View Subdivision

	ITE	Dwelling	Daily	AM Peak Hour	PM Peak Hour
Land Use	code	Units	Trips	Trips	Trips
Proposed Single Family Detached Housing	210	69	651	51 (in-13, out-38)	68 (in-43, out-25)
Existing Single Family Detached Housing	210	1	9	1 (in-0, out-1)	1 (in-1, out-0)
Net New Trips			642	50 (in-13, out-37)	67 (in-42, out-25)

The directional distribution of traffic generated by the development was assigned to the study area intersections. The distribution was based on the existing traffic volumes and previous traffic studies conducted in the area. The site traffic distribution and assignment diagrams are shown in Figures 7a and 7b.

Year 2025 Traffic Volumes

The year 2025 traffic volumes included a 3.0 percent per year compounded growth factor over the adjusted existing traffic volumes and in-process traffic. In-process traffic is traffic from developments that have been approved, but are not generating full build out traffic volumes. The in-process traffic was obtained from the City of La Center and is shown in Figures 5a, 5b and Appendix C. The in-process traffic from the La Center Middle School, Heritage Bldg. 'B' and Minit Management sites were added to the existing traffic counts that were conducted prior to 2022. In-process traffic from the Lockwood Meadows Subdivision was added to all of the existing traffic counts. The year 2025 traffic volumes without the project are shown in Figures 6a and 6b. The year 2025 traffic volumes with the project are shown in Figures 8a and 8b.

Peak Hour Traffic Operations

The scope of the transportation impact study was based on discussions with representatives from the City of La Center. Based on the discussions an analysis was conducted at the following intersections during the weekday AM and PM peak hours:

- (1) NE Lockwood Creek Road & NE 24th Avenue/site access
- (2) NE Lockwood Creek Road & E Spruce Avenue
- (3) NE Lockwood Creek Road & John Storm Avenue
- (4) NE Lockwood Creek Road & Highland Avenue
- (5) Aspen Avenue & E 4th Street
- (6) NW Pacific Hwy. & W 4th Street
- (7) NW La Center Road & NW Timmen Road

The study area intersections were analyzed to determine existing, year 2025 without project and year 2025 with project conditions. The assumption was made that the Asa's View Subdivision will be built out and occupied within a three year time period.

The intersection operational analysis was conducted using the procedures in the 2010 <u>Highway Capacity Manual</u>. These procedures describe the operation of an intersection in terms of its level of service (LOS). The LOS criteria ranges from "A", which indicates little, if any, delay to "F", which indicates that vehicles experience very long delays. The LOS criteria with the corresponding delay in seconds per vehicle is shown in Table 2 on page 4. The capacity analysis summary is shown in Table 3a on page 4 and Table 3b on page 5.

Table 2
Level of Service Criteria

Level of Service (LOS)	A	В	С	D	Е	F
Roundabouts						
Control Delay (seconds per vehicle)	≤10	>10 - 15	>15 - 25	>25 - 35	>35 - 50	>50
Unsignalized intersections						
Average Delay (seconds per vehicle)	≤10	>10 - 15	>15 - 25	>25 - 35	>35 - 50	>50

Table 3a Capacity Analysis Summary

	AM P	eak Hour	PM Pe	ak Hour
	LOS	Delay	LOS	Delay
		(sec/veh)		(sec/veh)
16				
NE Lockwood Creek Road & NE 24 th Aver				
Existing	A	9.5	В	10.6
Year 2025 w/o Project	A	9.9	В	10.6
Year 2025 with Project	В	11.3	В	10.7
NE Lockwood Creek Road & E Spruce Av	enue			
Existing	A	9.2	A	9.4
Year 2025 w/o Project	В	10.9	В	10.1
Year 2025 with Project	В	11.4	В	10.3
NE lockwood Creek Road & John Storm A	venue			
Existing	В	10.8	В	11.0
Year 2025 w/o Project	В	14.9	В	12.5
Year 2025 with Project	С	15.7	В	13.2
Highland Avenue & E 4 th Street				
Existing	F	83.7	С	20.7
Year 2025 w/o Project	F	>83.7	D	26.2
Year 2025 with Project	F	>83.7	D	28.6
Aspen Avenue & E 4 th Street				
Existing	С	16.7	В	11.5
Year 2025 w/o Project	C	19.3	В	12.7
Year 2025 with Project	C	20.6	В	13.5
1 car 2023 with Froject	C	20.0	Б	13.3
NW Pacific Avenue & W 4 th Street				
Existing	A	6.4	A	4.0
Year 2025 w/o Project	A	7.6	A	4.4
Year 2025 with Project	A	8.2	A	4.5

Table 3b
Capacity Analysis Summary (cont.)

	AM F	eak Hour	PM Pe	ak Hour
	LOS	Delay	LOS	Delay
		(sec/veh)		(sec/veh)
NW La Center Road & NW Timmen Road				
Existing	В	12.4	C	18.4
Year 2025 w/o Project	В	13.5	C	20.3
Year 2025 with Project	В	13.8	С	21.4

The City of La Center has adopted LOS "E" as the minimum acceptable performance at city intersections for stop controlled intersections. Based on the results of the capacity analysis this LOS will be met with build out of the Asa's View Subdivision with the exception of the NE Lockwood Creek Road/Highland Avenue intersection. This intersection is operating at LOS "F" during the AM peak hour. The LOS is attributed to vehicles on the northbound approach. The LOS computer printouts are included in Appendix E.

Pedestrian/Bicycle/Transit Considerations

Sidewalks will be provided for along the site frontage of Lockwood Creek Road. There are no existing or planned bike lanes. The site is not served by public transit service.

Sight Distance

Sight distance was measured at the site access onto Lockwood Creek Road. The measured intersection sight distance was over 300 feet when looking towards the east. Based on the criteria in AASHT0, A Policy on Geometric Design of Highways and Streets, 2011 and the posted speed limit of 25 mph on Lockwood Creek Road the recommended intersection sight distance is 280 feet. Therefore, the sight distance requirement is met. The measured intersection sight distance when looking towards the west was 225 feet and is obstructed by vegetation along the site frontage. The removal of the vegetation would improve the sight lines to over 300 feet meeting the criteria in AASHTO. This will occur with development of the site.

Turn Lanes

A left turn lane improves safety and increases the capacity of the roadway by reducing the speed differential between the through and left turning vehicles. Based on the low volume of vehicles entering the site from the east during the AM and PM peak hours a left turn lane is not justified as based on volumes.

Transportation Improvements

The Breeze Creek Culvert Replacement and 4th Street Widening Project is identified in the City of La Center's Capital Facilities Plan. Discussions are ongoing regarding improvements at the Highland Avenue/E 4th Street intersection. This intersection is operating at LOS "F" during the AM peak hour when school is in session. The main discussions involve installing a traffic signal or roundabout. The project is discussed in Appendix D.

Traffic Signal Warrant Analysis

A traffic signal warrant analysis using the tables and charts from the 2009 Manual on Uniform Traffic Control Devices (MUTCD) was conducted at the Highland Avenue/E 4th Street intersection. This intersection is operating at LOS "F" under existing conditions during the AM peak hour for vehicles approaching from the south. The signal warrant analysis was based on the year 2025 traffic conditions with project. Based on the traffic signal warrant analysis a traffic signal is not justified. The signal warrant analysis is shown in Table 4.

Table 4

Traffic Signal Warrant Analysis

Highland Avenue & E 4th Street, Year 2025 with Project Conditions, AM Peak hour

		Volumes ./Hr.)	Projected (Veh		
MUTCD Traffic Signal Warrant (1)	Major Street	Minor Street	Major Street	Minor Street	Warrant Met
1. Condition A – Minimum Vehicular Volume (3)	500	200	350 (2)	179 (2)	NO
2. Condition B – Interruption of Continuous Traffic (3)	750	100	525 (2)	179 (2)	NO
3. Peak Hour	Figure 4C-	3. Warrant	3, (MUTCI	0)	NO

- (1) Assumption made that two lanes are on minor street approach prior to traffic signal considerations.
- (2) Volumes assumed to be 70% of the AM Peak Hour volumes.
- (3) Warrant met if 1A or 1B is met.

Collision Data

Collision data was obtained from the Washington State Department of Transportation (WSDOT) for the most recent three years of available data. Based on the data only two accidents have been reported at the study area intersections. One accident occurred at the NE Lockwood Creek Road/Spruce Avenue intersection and involved a vehicle making an improper passing maneuver. The other accident was at the NW Pacific Avenue/W 4th street intersection and involved a vehicle going in the wrong direction. There were no injuries in either accident. The collision data is included in Appendix B.

CONCLUSIONS AND RECOMMENDATIONS

The Asa's View Subdivision is anticipated to generate 50 trips during the AM peak hour and 67 trips during the PM peak hour. This is based on a development consisting of 69 single family homes.

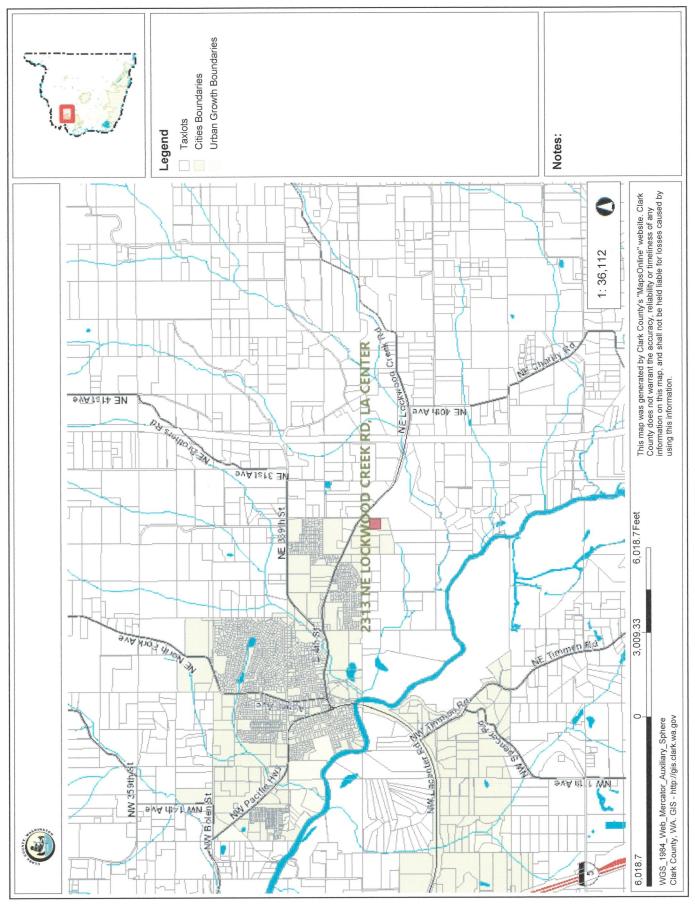
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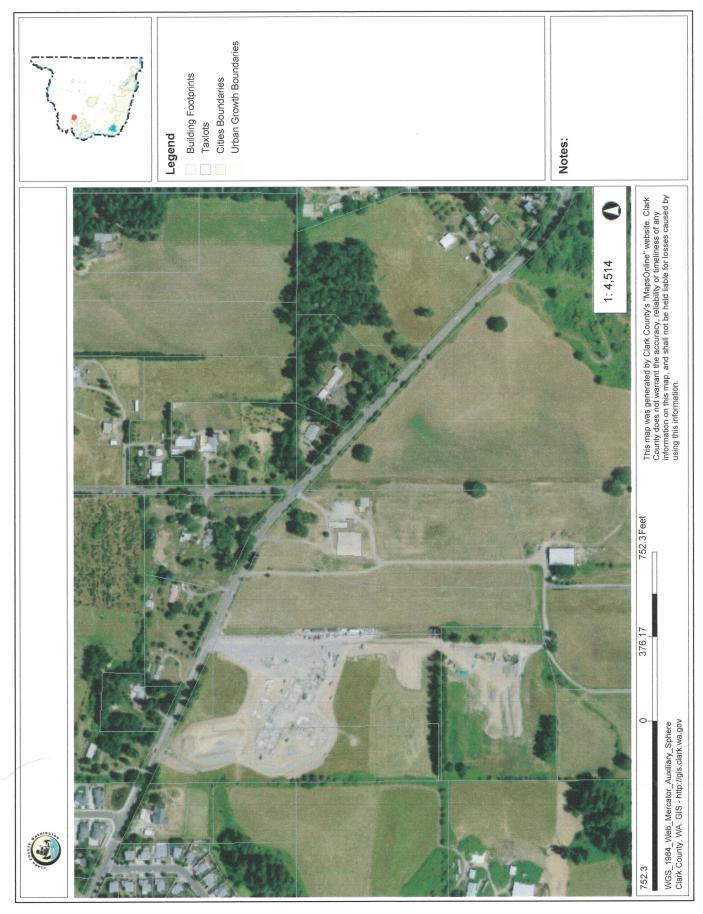
All of the study area intersections are operating at acceptable levels with the exception of the Highland Avenue/E 4th Street intersection. This intersection is operating at LOS "F" during the AM peak hour for vehicles approaching E 4th Street from the south. The failing condition would be mitigated by installing a traffic signal or roundabout. However, mitigating the failing condition is not proposed with the development of the Asa's View Subdivision for several reasons:

- 1. The failing approach is the northbound approach and the Asa's View Subdivision will add no traffic to this approach.
- 2. The issue of installing a roundabout or traffic signal has been identified, however funding is not available. The comments in the PBS report as identified in Appendix D stated that a do nothing alternative should be chosen until funds are available.
- 3. The Capital Facilities Plan suggested that future street connections in the area will alleviate motor vehicle demand in the area and improve the operation of the intersection.
- 4. A traffic signal is not warranted at the intersection.

Adequate sight distance should be maintained at the site access onto Lockwood Creek Road. Obstructions by vegetation, signs or other objects should not be allowed.

No additional transportation improvements or traffic control devices were identified to accommodate the development.







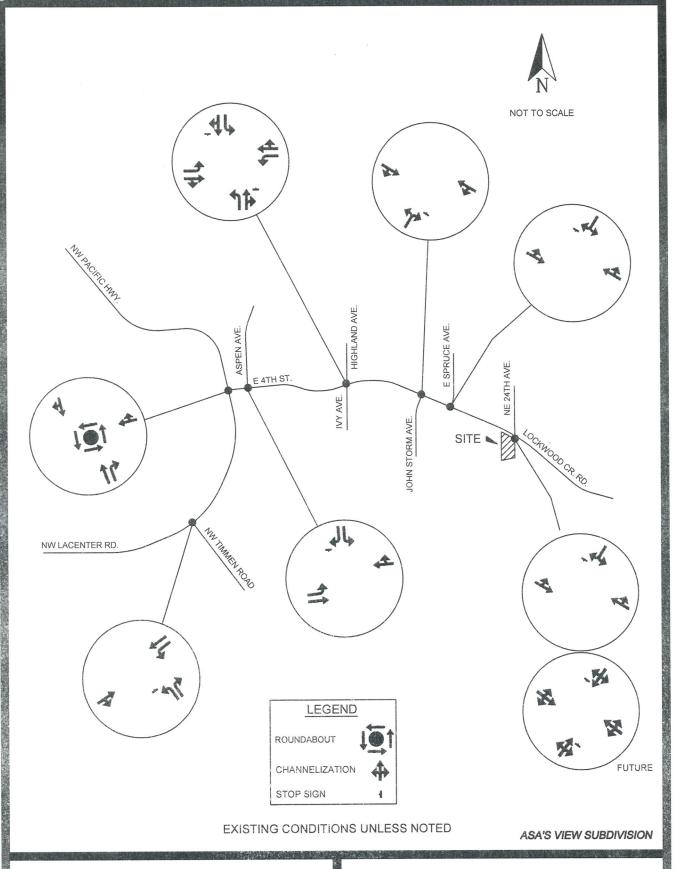


FIGURE 2
LANE CONFIGURATIONS

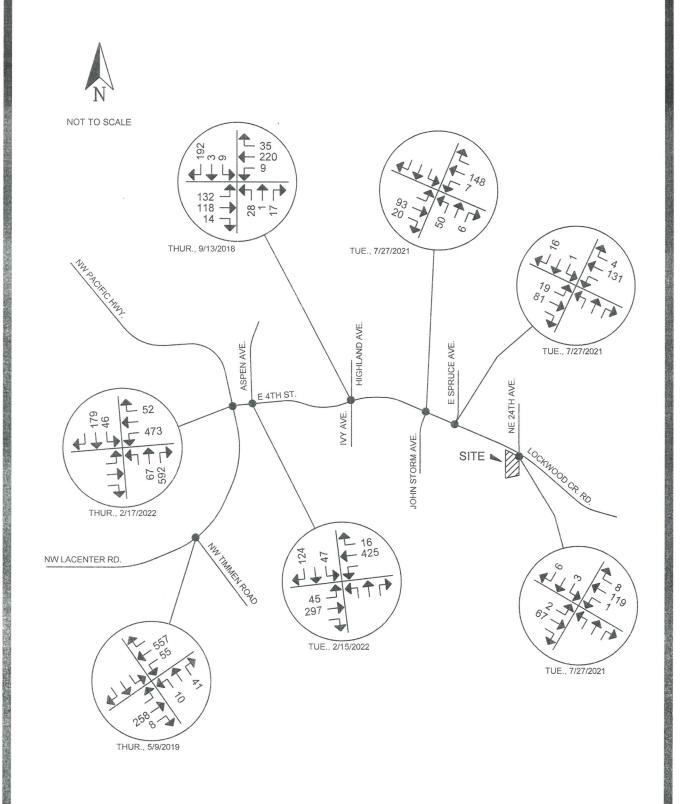


FIGURE 3a
EXISTING TRAFFIC VOLUMES
AM PEAK HOUR

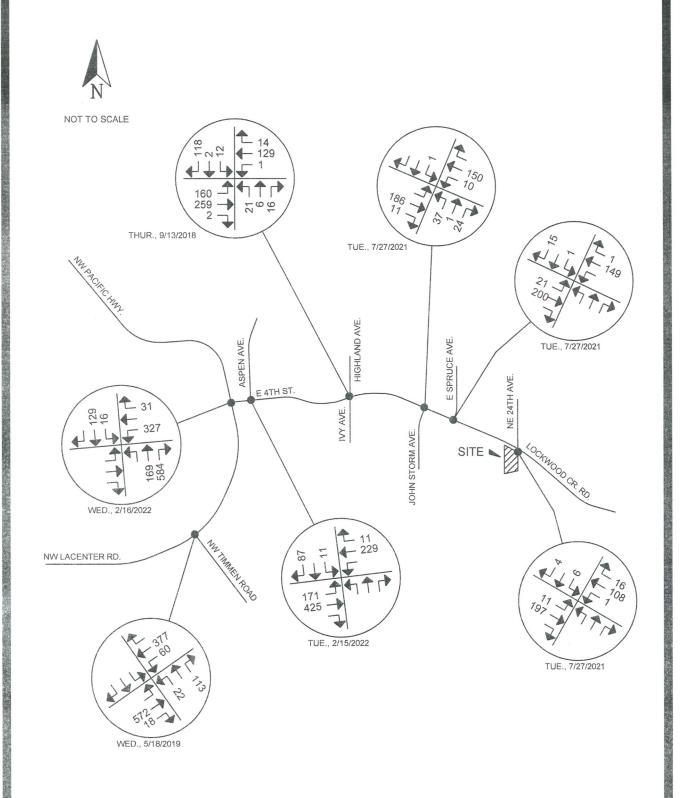


FIGURE 3b EXISTING TRAFFIC VOLUMES PM PEAK HOUR

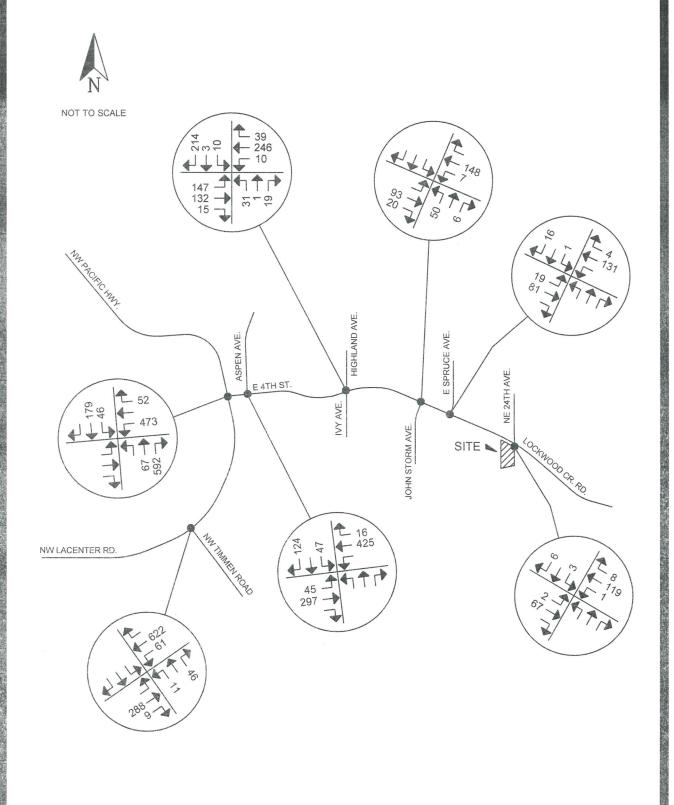


FIGURE 4a
ADJUSTED EXISTING TRAFFIC VOLUMES
AM PEAK HOUR

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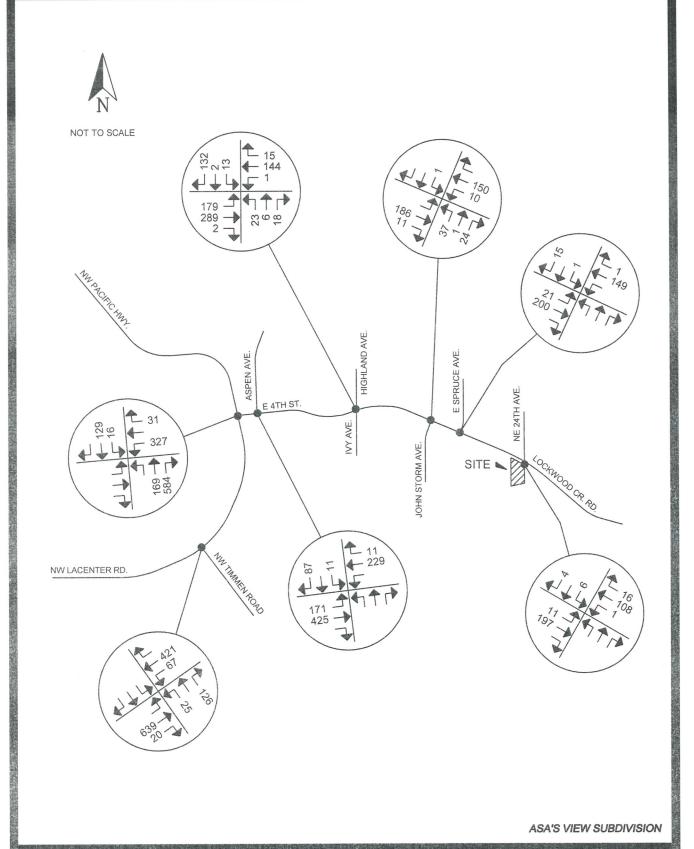


FIGURE 4b
ADJUSTED EXISTING TRAFFIC VOLUMES
PM PEAK HOUR

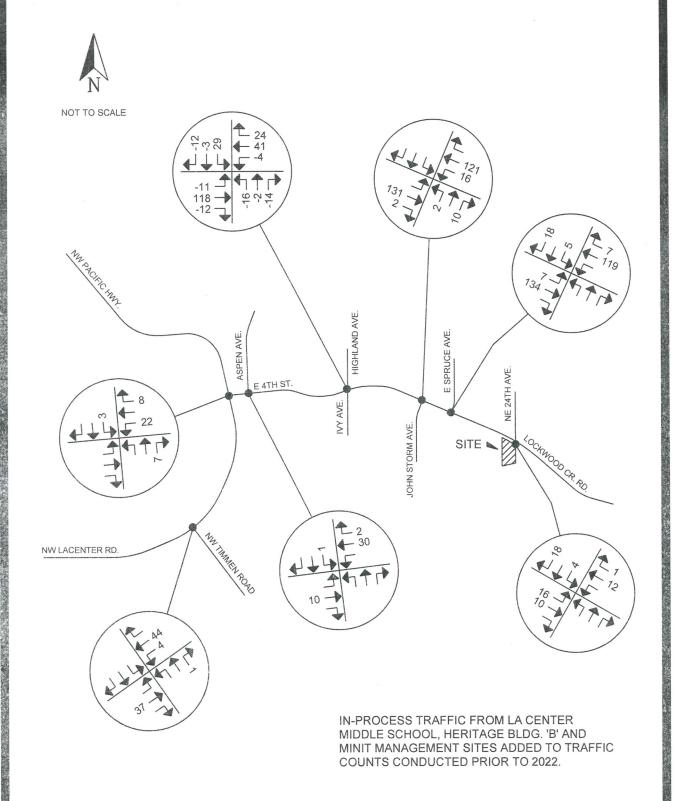


FIGURE 5a IN-PROCESS TRAFFIC AM PEAK HOUR

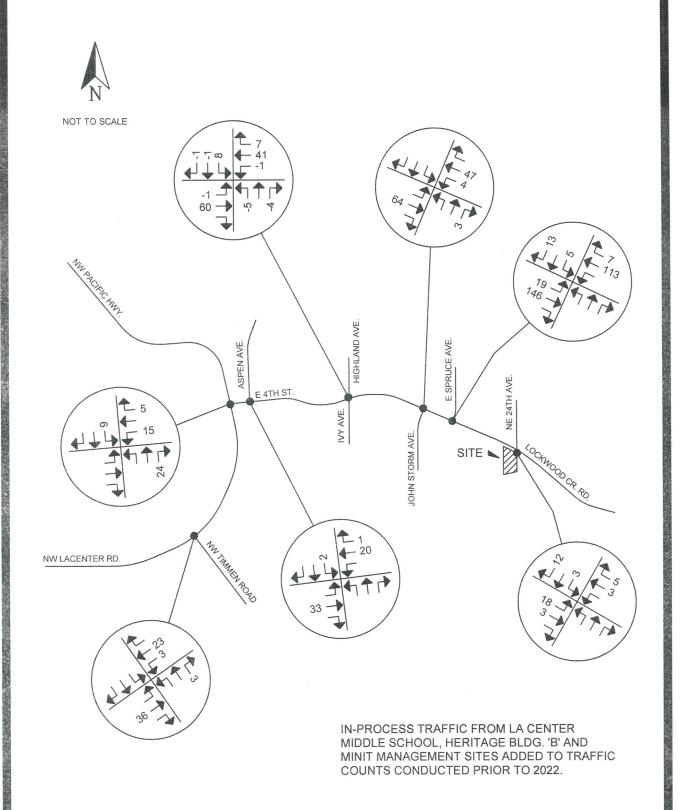


FIGURE 5b IN-PROCESS TRAFFIC PM PEAK HOUR

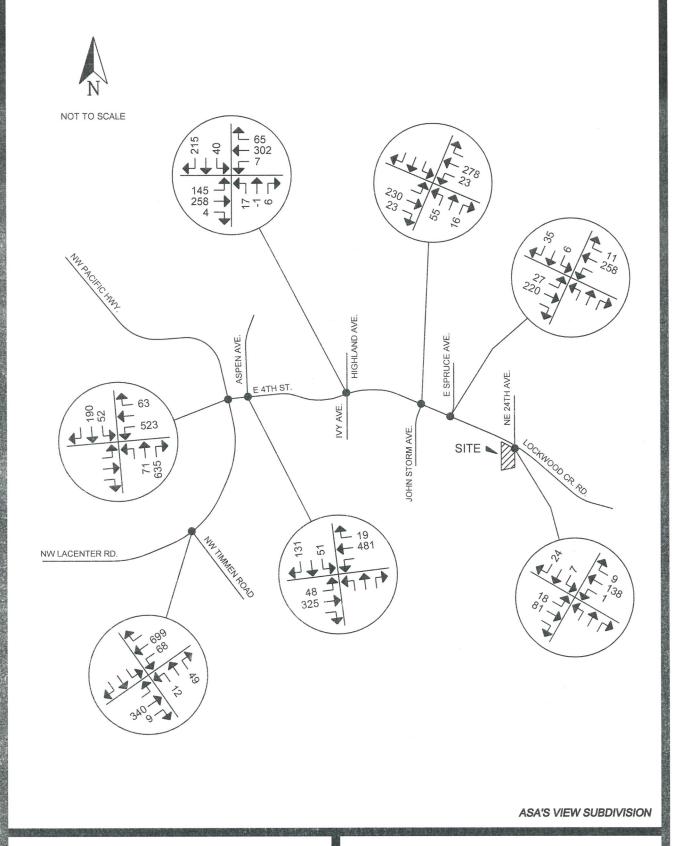


FIGURE 6a YEAR 2025 TRAFFIC VOLUMES W/O PROJECT, AM PEAK HOUR

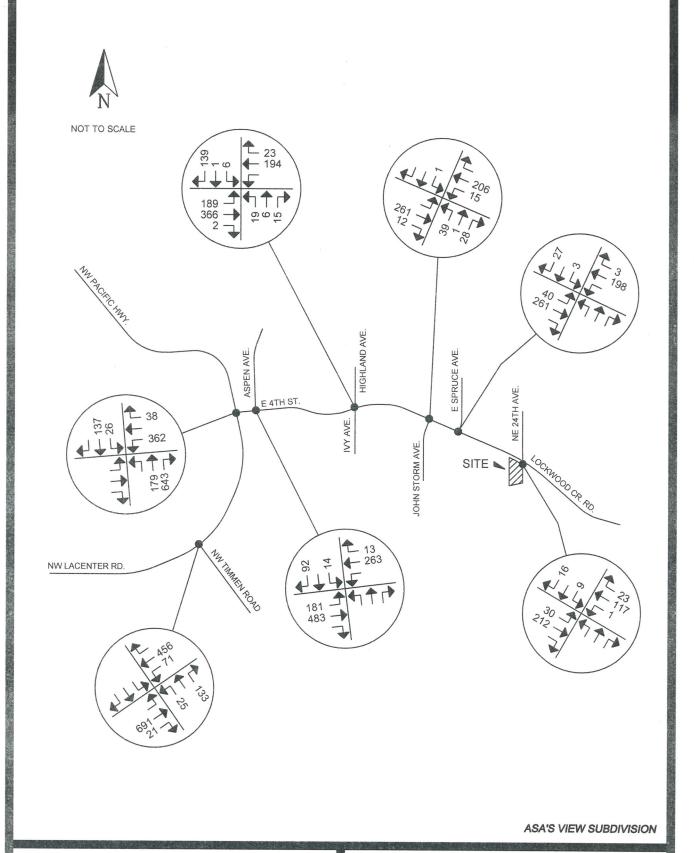


FIGURE 6b YEAR 2025 TRAFFIC VOLUMES W/O PROJECT, PM PEAK HOUR

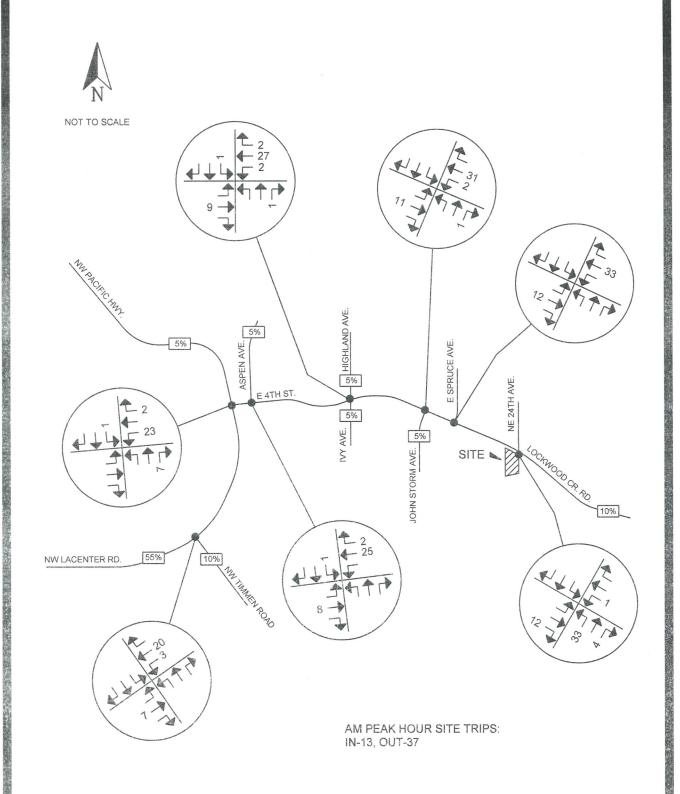


FIGURE 7a SITE TRAFFIC DISTRIBUTION/ASSIGNMENT AM PEAK HOUR

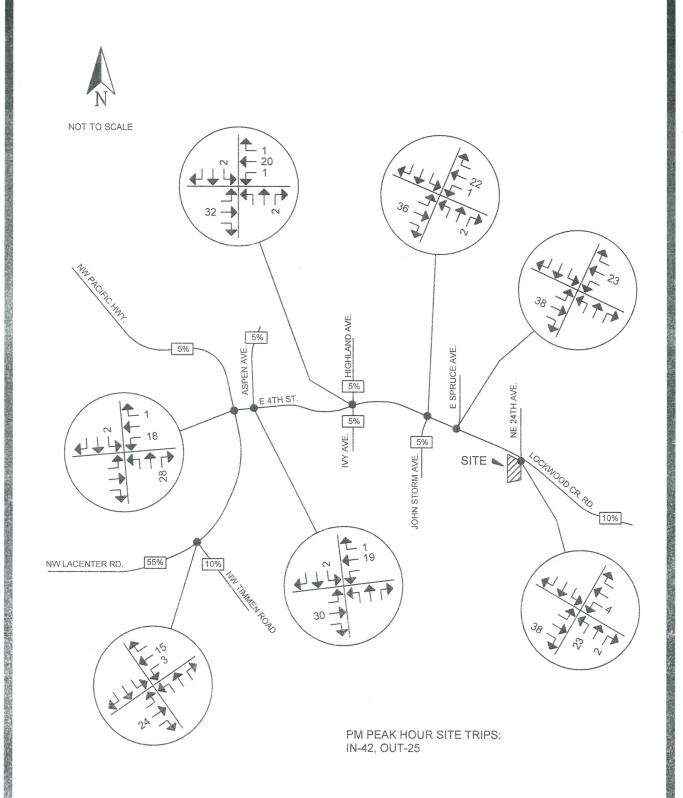


FIGURE 7b SITE TRAFFIC DISTRIBUTION/ASSIGNMENT PM PEAK HOUR KELLY ENGINEERING

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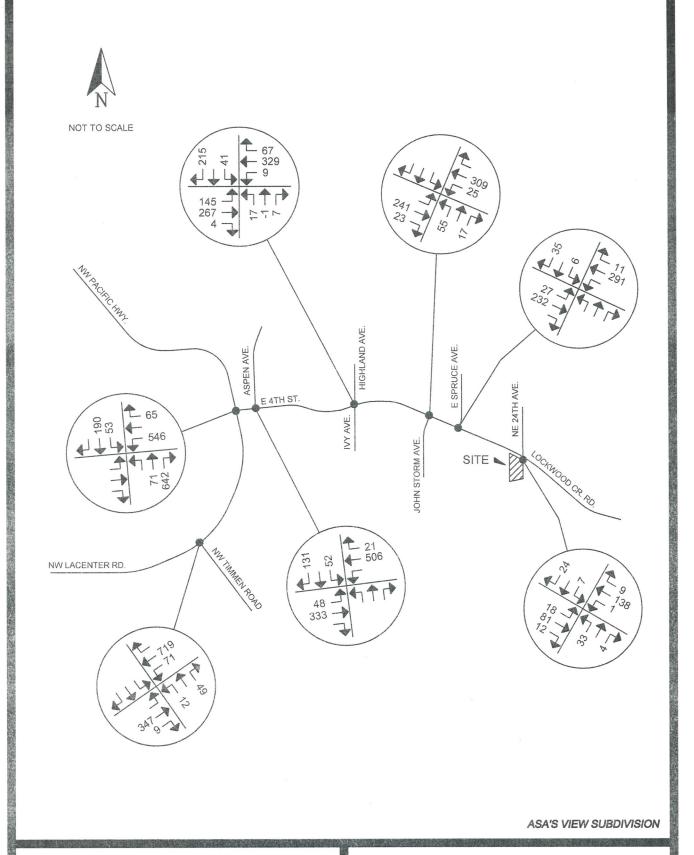


FIGURE 8a YEAR 2025 TRAFFIC VOLUMES WITH PROJECT, AM PEAK HOUR

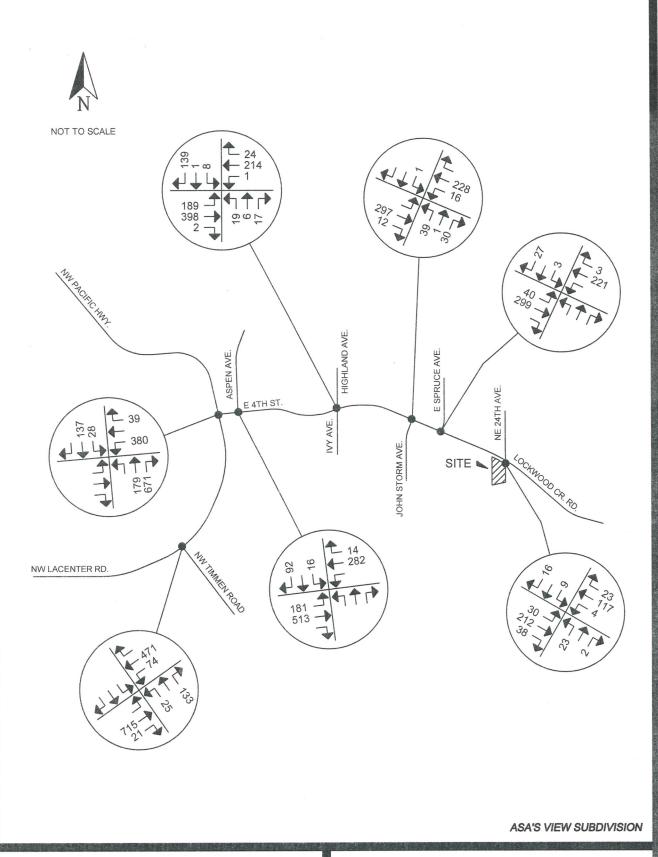


FIGURE 8b YEAR 2025 TRAFFIC VOLUMES WITH PROJECT, PM PEAK HOUR

APPENDIX A RAW TRAFFIC COUNT DATA



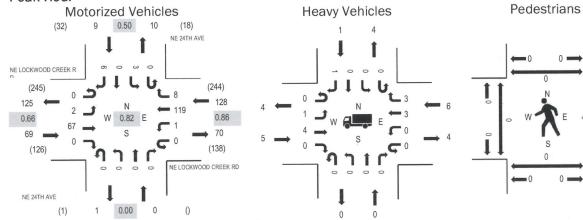
(303) 216-2439 www.alltrafficdata.net Location: 3 NE 24TH AVE & NE LOCKWOOD CREEK RD AM

Date: Tuesday, July 27, 2021

Peak Hour: 08:00 AM - 09:00 AM

Peak 15-Minutes: 08:35 AM - 08:50 AM

Peak Hour



Note: Total study counts contained in parentheses.

	HV%	PHF
EB	7.2%	0.66
WB	4.7%	0.86
NB	0.0%	0.00
SB	11.1%	0.50
All	5.8%	0.82

Traffic Counts - Motorized Vehicles

Interval	NE L		OD CRE	EK RD	NE L		OD CRE bound	EK RD			TH AVE			NE 24T South	bound			Rolling
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	Total	Hour
7:00 AM	0	0	4	0	0	0	6	0	0	0	0	0	0	1	0	0	11	196
7:05 AM	0	0	3	0	0	0	6	0	0	0	0	0	0	0	0	0	9	200
7:10 AM	0	0	1	0	0	0	13	0	0	0	0	0	0	0	0	0	14	202
7:15 AM	0	1	6	0	0	0	6	0	0	0	0	0	0	0	0	1	14	202
7:20 AM	0	1	4	0	0	0	20	0	0	0	0	0	0	1	0	0	26	199
7:25 AM	0	1	6	0	0	0	7	0	0	0	0	0	0	1	0	1	16	197
7:30 AM	0	0	2	0	0	0	9	1	0	0	0	0	0	3	. 0	2	17	203
7:35 AM	0	0	12	0	0	0	11	0	0	0	0	0	0	3	0	1	27	197
7:40 AM	0	0	2	0	0	0	8	0	0	0	0	0	0	2	0	1	13	190
7:45 AM	0	1	4	0	0	0	10	1	0	0	0	0	0	3	0	0	19	195
7:50 AM	0	0	3	0	0	0	10	0	0	0	0	0	0	0	0	1	14	201
7:55 AM	0	1	5	0	0	0	7	1	0	0	0	0	0	2	0	0	16	202
8:00 AM	0	0	5	0	0	0	9	0	0	0	0	0	0	0	0	1	15	206
8:05 AM	0	0	3	0	0	0	7	1	0	0	0	0	0	0	0	0	11	
8:10 AM	0	0	3	0	0	0	10	1	0	0	0	0	0	0	0	0	14	
8:15 AM	0	0	2	0	0	1	6	0	0	0	0	0	0	0	0	2	11	
8:20 AM	0	1	6	0	0	0	16	1	0	0	0	0	0	0	0	0	24	
8:25 AM	0	0	7	0	0	0	12	1	0	0	0	0	0	0	0	2	22	
8:30 AM	0	0	5	0	0	0	6	0	0	0	0	0	0	0	0	0	11	
8:35 AM	0	0	10	0	0	0	9	1	0	0	0	0	0	0	0	0	20	
8:40 AM	0	1	6	0	0	0	11	0	0	0	0	0	0	0	0	0	18	
8:45 AM	0	0	9	0	0	0	13	1	0	0	0	0	0	2	0	0	25	
8:50 AM	0	0	4	0	0	0	9	1	0	0	0	0	0	1	0	0	15	
8:55 AM	0	0	7	0	0	0	11	1	0	0	0	0	0	0	0	1	20	
Count Total	0	7	119	0	0	1	232	11	0	0	0	0	0	19	0	13	402	
Peak Hour	0	2	67	0	0	1	119	8	0	0	0	0	0	3	0	6	206	3

Traffic Counts - Heavy Vehicles, Bicycles on Road, and Pedestrians/Bicycles on Crosswalk

Interval		Hea	avy Vehicle	es		Interval						Interval						
Start Time	EB	NB	WB	SB	Total	Start Time	EB	NB	WB	SB	Total	Start Time	EB	NB	WB	SB	Total	
7:00 AM	0	0	0	0	0	7:00 AM	0	0	0	0	0	7:00 AM	0	0	0	0	0	
7:05 AM	0	0	2	0	2	7:05 AM	0	0	0	0	0	7:05 AM	0	0	0	0	0	
7:10 AM	0	0	0	0	0	7:10 AM	0	0	0	0	0	7:10 AM	0	0	0	0	0	
7:15 AM	0	0	0	0	0	7:15 AM	0	0	0	0	0	7:15 AM	0	0	0	0	0	
7:20 AM	1	0	1	0	2	7:20 AM	0	0	0	0	0	7:20 AM	0	0	0	0	0	
7:25 AM	0	0	0	0	0	7:25 AM	0	0	0	0	0	7:25 AM	0	0	0	0	0	
7:30 AM	0	0	0	0	0	7:30 AM	0	0	0	0	0	7:30 AM	0	0	0	0	0	
7:35 AM	1	0	0	0	1	7:35 AM	0	0	0	0	0	7:35 AM	0	0	0	0	0	
7:40 AM	0	0	0	0	0	7:40 AM	0	0	0	0	0	7:40 AM	0	0	0	0	0	
7:45 AM	0	0	0	0	0	7:45 AM	0	0	0	0	0	7:45 AM	0	0	0	0	0	
7:50 AM	0	0	0	0	0	7:50 AM	0	0	0	0	0	7:50 AM	0	0	0	0	0	
7:55 AM	0	0	0	0	0	7:55 AM	0	0	0	0	0	7:55 AM	0	0	0	0	0	
8:00 AM	1	0	0	0	1	8:00 AM	0	0	0	0	0	8:00 AM	0	0	0	0	0	
8:05 AM	0	0	1	0	1	8:05 AM	0	0	0	0	0	8:05 AM	0	0	0	0	0	
8:10 AM	0	0	1	0	1	8:10 AM	0	0	0	0	0	8:10 AM	0	0	0	0	0	
8:15 AM	0	0	0	1	1	8:15 AM	0	0	0	0	0	8:15 AM	0	0	0	0	0	
8:20 AM	1	0	0	0	1	8:20 AM	0	0	0	0	0	8:20 AM	0	0	0	0	0	
8:25 AM	0	0	2	0	2	8:25 AM	0	0	0	0	0	8:25 AM	0	0	0	0	0	
8:30 AM	0	0	0	0	0	8:30 AM	0	0	0	0	0	8:30 AM	0	0	0	0	0	
8:35 AM	1	0	1	0	2	8:35 AM	0	0	0	0	0	8:35 AM	0	0	0	0	0	
8:40 AM	1	0	0	0	1	8:40 AM	1	0	0	0	1	8:40 AM	0	0	0	0	0	
8:45 AM	0	0	0	0	0	8:45 AM	0	0	0	0	0	8:45 AM	0	. 0	0	0	0	
8:50 AM	1	0	1	0	2	8:50 AM	0	0	0	0	0	8:50 AM	0	0	0	0	0	
8:55 AM	0	0	0	0	0	8:55 AM	0	0	0	0	0	8:55 AM	0	0	0	0	0	
Count Total	7	0	9	1	17	Count Total	1	0	0	0	1	Count Total	0	0	0	0	0	
Peak Hour	5	0	6	1	12	Peak Hour	1	0	0	0	1	Peak Hour	0	0	0	0	0	



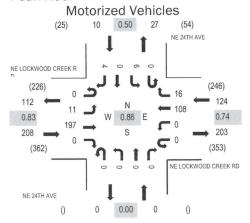
(303) 216-2439 www.alltrafficdata.net Location: 3 NE 24TH AVE & NE LOCKWOOD CREEK RD PM

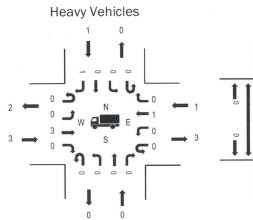
Date: Tuesday, July 27, 2021

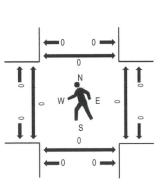
Peak Hour: 04:30 PM - 05:30 PM

Peak 15-Minutes: 04:35 PM - 04:50 PM

Peak Hour







Pedestrians

Note: Total study counts contained in parentheses.

	HV%	PHF
EB	1.4%	0.83
WB	0.8%	0.74
NB	0.0%	0.00
SB	10.0%	0.50
All	1.5%	0.86

Traffic Counts - Motorized Vehicles

manno odanico	141000	11200	* 01110	,,,,,														
	NE L		OD CRE	EK RD	NE L		OD CRE	EK RD			TH AVE				TH AVE			D 111
Interval			bound				bound				bound				nbound			Rolling
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	Total	Hour
4:00 PM	0	2	14	0	0	0	11	1	0	0	0	0	0	0	0	0	28	313
4:05 PM	0	2	10	0	0	0	10	1	0	0	0	0	0	1	0	0	24	316
4:10 PM	0	0	11	0	0	0	15	1	0	0	0	0	0	0	0	0	27	316
4:15 PM	0	0	12	0	0	0	4	0	0	0	0	0	0	1	0	0	17	308
4:20 PM	0	1	11	0	0	0	9	1	0	0	0	0	0	0	0	0	22	322
4:25 PM	0	0	12	0	0	0	6	2	0	0	0	0	0	0	0	0	20	334
4:30 PM	0	0	22	0	0	0	5	1	0	0	0	0	0	0	0	1	29	342
4:35 PM	0	0	19	0	0	0	15	2	0	0	0	0	0	1	0	0	37	335
4:40 PM	0	0	15	0	0	0	12	3	0	0	0	0	0	0	0	1	31	329
4:45 PM	0	0	19	0	0	0	9	2	0	0	0	0	0	0	0	1	31	328
4:50 PM	0	1	17	0	0	0	8	2	0	0	0	0	0	1	0	0	29	319
4:55 PM	0	3	6	0	0	0	6	1	0	0	0	0	0	1	0	1	18	317
5:00 PM	0	0	19	0	0	0	9	2	0	0	0	0	0	1	0	0	31	320
5:05 PM	0	0	12	0	0	0	11	0	0	0	0	0	0	1	0	0	24	
5:10 PM	0	1	11	0	0	0	6	0	0	0	0	0	0	1	0	0	19	
5:15 PM	0	3	20	0	0	0	8	0	0	0	0	0	0	0	0	0	31	
5:20 PM	0	1	20	0	0	0	10	3	0	0	0	0	0	0	0	0	34	
5:25 PM	0	2	17	0	0	0	9	0	0	0	0	0	0	0	0	0	28	
5:30 PM	0	1	11	0	0	0	6	1	0	0	0	0	0	2	0	1	22	
5:35 PM	0	2	15	0	0	0	14	0	0	0	0	0	0	0	0	0	31	
5:40 PM	0	1	12	0	0	0	10	2	0	0	0	0	0	1	0	4	30	
5:45 PM	0	3	9	0	0	0	5	1	0	0	0	0	0	4	0	0	22	
5:50 PM	0	1	13	0	0	0	11	2	0	0	0	0	0	0	0	0	27	
5:55 PM	0	0	11	0	0	0	7	2	0	0	0	0	0	0	0	1	21	
Count Total	0	24	338	0	0	0	216	30	0	0	0	0	0	15	0	10	633	
Peak Hour	0	11	197	0	0	0	108	16	0	0	0	0	0	6	0	4	342	2

Traffic Counts - Heavy Vehicles, Bicycles on Road, and Pedestrians/Bicycles on Crosswalk

Interval		Hea	vy Vehicle	es		Interval	Bicycles on Roadway					Interval	Ped	destrians/E	Bicycles on	Crosswa	lk
Start Time	EB	NB	WB	SB	Total	Start Time	EB	NB	WB	SB	Total	Start Time	EB	NB	WB	SB	Total
4:00 PM	0	0	0	0	0	4:00 PM	0	0	0	0	0	4:00 PM	0	0	0	0	0
4:05 PM	1	0	0	0	1	4:05 PM	0	0	0	0	0	4:05 PM	0	0	0	0	0
4:10 PM	0	0	0	0	0	4:10 PM	0	0	0	0	0	4:10 PM	0	0	0	0	0
4:15 PM	0	0	0	0	0	4:15 PM	0	0	0	0	0	4:15 PM	0	0	0	0	0
4:20 PM	0	0	0	0	0	4:20 PM	0	0	0	0	0	4:20 PM	0	0	0	0	0
4:25 PM	0	0	0	0	0	4:25 PM	0	0	0	0	0	4:25 PM	0	0	0	0	0
4:30 PM	0	0	0	1	1	4:30 PM	0	0	0	0	0	4:30 PM	0	0	0	0	0
4:35 PM	0	0	0	0	0	4:35 PM	0	0	0	0	0	4:35 PM	0	0	0	0	0
4:40 PM	1	0	0	0	. 1	4:40 PM	0	0	0	0	0	4:40 PM	0	0	0	0	0
4:45 PM	0	0	0	0	0	4:45 PM	0	0	0	0	0		0	0	0	0	0
4:50 PM	1	0	0	0	1	4:50 PM	0	0	0	0	0	4:50 PM	0	0	0	0	0
4:55 PM	0	0	0	0	0	4:55 PM	0	0	0	0	0	4:55 PM	0	0	0	0	0
5:00 PM	0	0	0	0	0	5:00 PM	0	0	0	0	0	5:00 PM	0	0	0	0	0
5:05 PM	0	0	0	0	0	5:05 PM	1	0	0	0	1	5:05 PM	0	0	0	0	0
5:10 PM	1	0	0	0	1	5:10 PM	0	0	0	0	0		0	0	0	0	0
5:15 PM	0	0	0	0	0	5:15 PM	0	0	0	0	0		0	0	0	0	0
5:20 PM	0	0	1	0	1	5:20 PM	0	0	0	0	0		0	0	0	0	0
5:25 PM	0	0	0	0	0	5:25 PM	0	0	0	0	0		0	0	0	0	0
5:30 PM	0	0	0	0	0	5:30 PM	0	0	0	0	0	5:30 PM	0	0	0	0	0
5:35 PM	0	0	0	0	0	5:35 PM	0	0	0	0	0		0	0	0	0	0
5:40 PM	0	. 0	0	0	0	5:40 PM	0	0	0	0	0		0	0	0	0	0
5:45 PM	0	0	0	0	0	5:45 PM	0	0	0	0	0	***************************************	0	0	0	0	0
5:50 PM	0	0	0	0	0	5:50 PM	0	0	0	0	0	5:50 PM	0	0	0	0	0
5:55 PM	0	0	0	0	0	5:55 PM	0	0	0	0	0		0	0	0	0	0
Count Total	4	0	1	1	6	Count Total	1	0	0	0	1	Count Total	0	0	0	0	0
Peak Hour	3	0	1	- 1	5	Peak Hour	1	0	0	0	1	Peak Hour	0	0	0	0	0



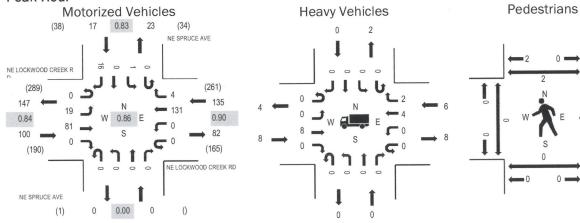
(303) 216-2439 www.alltrafficdata.net Location: 2 NE SPRUCE AVE & NE LOCKWOOD CREEK RD AM

Date: Tuesday, July 27, 2021

Peak Hour: 07:10 AM - 08:10 AM

Peak 15-Minutes: 07:50 AM - 08:05 AM

Peak Hour



Note: Total study counts contained in parentheses.

	HV%	PHF
EB	8.0%	0.84
WB	4.4%	0.90
NB	0.0%	0.00
SB	0.0%	0.83
All	5.6%	0.86

Traffic Counts - Motorized Vehicles

Interval	NE LO		OD CREI	EK RD	NE L		OD CRE	EK RD			UCE AVE		1		Rolling			
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	Total	Hour
7:00 AM	0	3	11	0	0	0	7	0	0	0	0	0	0	0	0	3	24	249
7:05 AM	0	1	6	0	0	1	3	0	0	0	0	0	0	0	0	2	13	251
7:10 AM	0	4	4	0	0	0	11	0	0	0	0	0	0	0	0	1	20	252
7:15 AM	0	0	6	0	0	0	10	0	0	0	0	0	0	0	0	2	18	250
7:20 AM	0	2	5	0	0	0	13	0	0	0	0	0	0	0	0	1	21	24
7:25 AM	0	0	4	0	0	0	18	0	0	0	0	0	0	0	0	1	23	244
7:30 AM	0	0	8	0	0	0	7	0	0	0	0	0	0	1	0	2	18	24
7:35 AM	0	2	13	0	0	0	13	0	0	0	0	0	0	0	0	1	29	249
7:40 AM	0	1	6	0	0	0	10	0	0	0	0	0	0	0	0	1	18	238
7:45 AM	0	3	4	0	0	0	9	0	0	0	0	0	0	0	0	2	18	24
7:50 AM	0	2	6	0	0	0	12	2	0	0	0	0	0	0	0	1	23	24
7:55 AM	0	2	11	0	0	0	7	2	0	0	0	0	0	0	0	2	24	24
8:00 AM	0	2	8	0	0	0	15	0	0	0	0	0	0	0	0	1	26	24
8:05 AM	0	1	6	0	0	0	6	0	0	0	0	0	0	0	0	1	14	
8:10 AM	0	2	3	0	0	0	11	0	0	0	0	0	0	0	0	2	18	
8:15 AM	0	1	2	0	0	0	11	1	0	0	0	0	0	0	0	0	15	
8:20 AM	0	0	6	0	0	0	10	0	0	0	0	0	0	0	0	2	18	
8:25 AM	0	0	8	0	0	0	13	0	0	0	0	0	0	1	0	3	25	
8:30 AM	0	0	8	0	0	0	13	0	0	0	0	0	0	0	0	0	21	
8:35 AM	0	1	9	0	0	0	7	0	0	0	0	0	0	0	0	1	18	
8:40 AM	0	0	8	0	0	0	12	0	0	0	0	0	0	0	0	1	21	
8:45 AM	0	0	9	0	0	0	10	0	0	0	0	0	0	0	0	1	20	
8:50 AM	0	0	5	0	0	0	14	0	0	0	0	0	0	0	0	2	21	
8:55 AM	0	0	7	0	0	0	11	2	0	0	0	0	0	0	0	3	23	
Count Total	0	27	163	0	0	1	253	7	0	0	0	0	0	2	0	36	489	_
Peak Hour	0	19	81	0	0	0	131	4	0	0	0	0	0	1	0	16	252	2

Traffic Counts - Heavy Vehicles, Bicycles on Road, and Pedestrians/Bicycles on Crosswalk

Interval		Hea	vy Vehicle	es		Interval		Bicycle	es on Road	dway		Interval	Pedestrians/Bicycles on Crosswalk				
Start Time	EB	NB	WB	SB	Total	Start Time	EB	NB	WB	SB	Total	Start Time	EB	NB	WB	SB	Total
7:00 AM	1	0	0	0	1	7:00 AM	0	0	0	0	0	7:00 AM	0	0	1	0	1
7:05 AM	0	0	1	0	1	7:05 AM	0	0	0	0	0	7:05 AM	0	0	0	0	0
7:10 AM	1	0	1	0	2	7:10 AM	0	0	0	0	0	7:10 AM	0	0	0	0	0
7:15 AM	0	0	0	0	0	7:15 AM	0	0	0	0	0	7:15 AM	0	0	0	0	0
7:20 AM	1	0	0	0	1	7:20 AM	0	0	0	0	0	7:20 AM	0	0	0	0	0
7:25 AM	0	0	1	0	1	7:25 AM	0	0	0	0	0	7:25 AM	0	0	0	0	0
7:30 AM	1	0	1	0	2	7:30 AM	0	0	0	0	0	7:30 AM	0	0	0	0	0
7:35 AM	2	0	0	0	2	7:35 AM	0	0	0	0	0	7:35 AM	0	0	0	0	0
7:40 AM	0	0	0	0	0	7:40 AM	0	0	0	0	0	7:40 AM	0	0	0	0	0
7:45 AM	0	0	0	0	0	7:45 AM	0	0	0	0	0	7:45 AM	0	0	0	0	0
7:50 AM	1	0	1	0	2	7:50 AM	0	0	0	0	0	7:50 AM	0	0	0	0	0
7:55 AM	1	0	1	0	2	7:55 AM	0	0	0	0	0	7:55 AM	0	0	0	2	2
8:00 AM	1	0	1	0	2	8:00 AM	0	0	0	0	0	8:00 AM	0	0	0	0	0
8:05 AM	0	0	0	0	0	8:05 AM	0	0	0	0	0	8:05 AM	0	0	0	0	0
8:10 AM	0	0	1	0	1	8:10 AM	0	0	0	0	0	8:10 AM	0	0	0	0	0
8:15 AM	0	0	2	0	2	8:15 AM	0	0	0	0	0		0	0	0	0	0
8:20 AM	1	0	0	0	1	8:20 AM	0	0	0	0	0		0	0	0	0	0
8:25 AM	0	0	0	0	0	8:25 AM	0	0	0	0	0	8:25 AM	0	0	0	0	0
8:30 AM	0	0	1	0	1	8:30 AM	0	0	0	0	0	***************************************	0	0	0	0	0
8:35 AM	2	0	1	0	3	8:35 AM	0	0	0	0	0	8:35 AM	2	0	0	2	4
8:40 AM	2	0	1	0	3	8:40 AM	1	0	0	0	1	0.107411	0	0	0	0	0
8:45 AM	0	0	0	0	0	8:45 AM	0	0	0	0	0	8:45 AM	0	0	0	0	0
8:50 AM	1	0	0	0	1	8:50 AM	0	0	0	0	0	8:50 AM	0	0	0	0	0
8:55 AM	0	0	0	1	1	8:55 AM	0	0	0	0	0		0	0	0	0	0
Count Total	15	0	13	1	29	Count Total	1	0	0	0	1	Count Total	2	0	1	4	7
Peak Hour	8	0	6	0	14	Peak Hour	0	0	0	0	0	Peak Hour	0	0	0	2	2



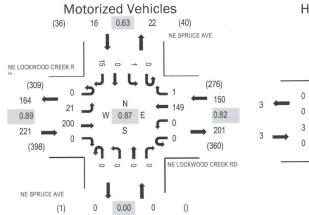
(303) 216-2439 www.alltrafficdata.net Location: 2 NE SPRUCE AVE & NE LOCKWOOD CREEK RD PM

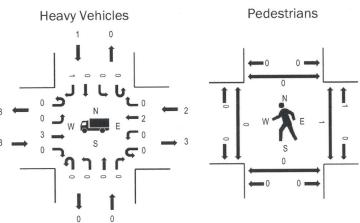
Date: Tuesday, July 27, 2021

Peak Hour: 04:30 PM - 05:30 PM

Peak 15-Minutes: 04:30 PM - 04:45 PM

Peak Hour





Note: Total study counts contained in parentheses.

	HV%	PHF
EB	1.4%	0.89
WB	1.3%	0.82
NB	0.0%	0.00
SB	6.3%	0.63
All	1.6%	0.87

Traffic Counts - Motorized Vehicles

Interval	NE LO		OD CREI	EK RD	NE L		OD CRE bound	EK RD			UCE AVE		NE SPRUCE AVE Southbound					Rolling
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	Total	Hour
4:00 PM	0	3	14	0	0	0	10	0	0	0	0	0	0	0	0	0	27	355
4:05 PM	0	2	10	0	0	0	8	0	0	0	0	0	0	0	0	3	23	361
4:10 PM	0	1	15	0	0	0	18	0	0	0	0	0	0	0	0	0	34	369
4:15 PM	0	0	15	0	0	0	8	0	0	0	0	0	0	0	0	3	26	36
4:20 PM	0	1	13	0	0	0	8	0	0	0	0	0	0	0	0	3	25	36
4:25 PM	0	1	7	0	0	0	11	0	0	0	0	0	0	0	0	4	23	38
4:30 PM	0	2	22	0	0	0	10	0	0	0	0	0	0	0	0	1	35	38
4:35 PM	0	1	18	0	0	0	25	0	0	0	0	0	0	0	0	2	46	38
4:40 PM	0	0	16	0	0	0	10	1	0	0	0	0	0	0	0	3	30	36
4:45 PM	0	3	13	0	0	0	10	0	0	0	0	0	0	0	0	3	29	35
4:50 PM	0	3	21	0	0	0	13	0	0	0	0	0	0	0	0	1	38	36
4:55 PM	0	1	10	0	0	0	8	0	0	0	0	0	0	0	0	0	19	35
5:00 PM	0	3	19	0	0	0	9	0	0	0	0	0	0	0	0	2	33	35
5:05 PM	0	2	11	0	0	0	17	0	0	0	0	0	0	1	0	0	31	
5:10 PM	0	3	11	0	0	0	10	0	0	0	0	0	0	0	0	1	25	
5:15 PM	0	1	21	0	0	0	11	0	0	0	0	0	0	0	0	0	33	
5:20 PM	0	2	24	0	0	0	11	0	0	0	0	0	0	0	0	1	38	
5:25 PM	0	0	14	0	0	0	15	0	0	0	0	0	0	0	0	1	30	
5:30 PM	0	0	18	0	0	0	7	0	0	0	0	0	0	0	0	3	28	
5:35 PM	0	0	16	1	0	0	11	0	0	0	0	0	0	0	0	1	29	
5:40 PM	0	0	10	0	0	0	15	0	0	0	0	0	0	1	0	0	26	
5:45 PM	0	5	11	0	0	0	13	0	0	0	0	0	0	0	0	1	30	
5:50 PM	0	3	15	0	0	0	10	0	0	0	0	0	0	0	0	0	28	
5:55 PM	0	2	14	0	0	0	7	0	0	0	0	0	0	0	0	1	24	
Count Total	0	39	358	1	0	0	275	1	0	0	0	0	0	2	0	34	710	
Peak Hour	0	21	200	0	0	0	149	1	0	0	0	0	0	1	0	15	387	•

Traffic Counts - Heavy Vehicles, Bicycles on Road, and Pedestrians/Bicycles on Crosswalk

Interval Heavy Vehicles Interval Bicycles on Roadway Interval Interval

Interval		Hea	avy Vehicl	es		Interval		Bicycle	es on Road	dway		Interval	Ped	destrians/E	Bicycles or	Crosswa	lk
Start Time	EB	NB	WB	SB	Total	Start Time	EB	NB	WB	SB	Total	Start Time	EB	NB	WB	SB	Total
4:00 PM	0	0	0	0	0	4:00 PM	0	0	0	0	0	4:00 PM	0	0	0	0	0
4:05 PM	0	0	0	0	0	4:05 PM	0	0	0	0	0	4:05 PM	0	0	0	0	0
4:10 PM	1	0	0	0	1	4:10 PM	0	0	0	0	0	4:10 PM	1	0	0	0	1
4:15 PM	0	0	0	0	0	4:15 PM	0	0	0	0	0	4:15 PM	0	0	0	0	0
4:20 PM	0	0	0	0	0	4:20 PM	0	0	0	0	0	4:20 PM	0	0	0	0	0
4:25 PM	0	0	0	2	2	4:25 PM	0	0	0	0	0	4:25 PM	0	0	0	0	0
4:30 PM	0	0	0	0	0	4:30 PM	0	0	0	0	0	4:30 PM	0	0	1	0	1
4:35 PM	0	0	1	0	1	4:35 PM	0	0	0	0	0	4:35 PM	0	0	0	0	0
4:40 PM	1	0	0	0	1	4:40 PM	0	0	0	0	0	4:40 PM	0	0	0	0	0
4:45 PM	0	0	0	1	1	4:45 PM	0	0	0	0	0	4:45 PM	0	0	0	0	0
4:50 PM	1	0	0	0	1	4:50 PM	0	0	0	0	0	4:50 PM	0	0	0	0	0
4:55 PM	0	0	0	0	0	4:55 PM	0	0	0	0	0	4:55 PM	0	0	0	0	0
5:00 PM	0	0	0	0	0	5:00 PM	0	0	0	0	0	5:00 PM	0	0	0	0	0
5:05 PM	0	0	0	0	0	5:05 PM	0	0	0	0	0	5:05 PM	0	0	0	0	0
5:10 PM	1	0	0	0	1	5:10 PM	1	0	0	0	1	5:10 PM	0	0	0	0	0
5:15 PM	0	0	0	0	0	5:15 PM	0	0	0	0	0	5:15 PM	0	0	0	0	0
5:20 PM	0	0	1	0	1	5:20 PM	0	0	0	0	0	5:20 PM	0	0	0	0	0
5:25 PM	0	0	0	0	0	5:25 PM	0	0	0	0	0	5:25 PM	0	0	0	0	0
5:30 PM	0	0	0	0	0	5:30 PM	0	0	0	0	0	5:30 PM	0	0	0	0	0
5:35 PM	0	0	0	0	0	5:35 PM	0	0	0	0	0	5:35 PM	0	0	0	0	0
5:40 PM	0	0	0	0	0	5:40 PM	0	0	0	0	0	5:40 PM	0	0	0	0	0
5:45 PM	0	0	0	0	0	5:45 PM	0	0	0	0	0	5:45 PM	0	0	0	0	0
5:50 PM	0	0	0	0	0	5:50 PM	0	0	0	0	0	5:50 PM	0	0	0	0	0
5:55 PM	0	0	0	0	0	5:55 PM	0	0	0	0	0	5:55 PM	0	0	0	0	0
Count Total	4	0	2	3	9	Count Total	1	0	0	0	1	Count Total	1	0	1	0	2
Peak Hour	3	0	2	1	6	Peak Hour	1	0	0	0	1	Peak Hour	0	0	1	0	1



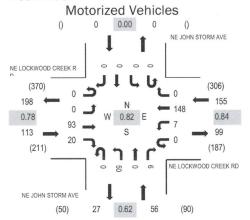
(303) 216-2439 www.alltrafficdata.net Location: 1 NE JOHN STORM AVE & NE LOCKWOOD CREEK RD AM

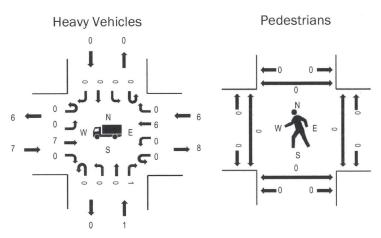
Date: Tuesday, July 27, 2021

Peak Hour: 07:10 AM - 08:10 AM

Peak 15-Minutes: 07:25 AM - 07:40 AM

Peak Hour





Note: Total study counts contained in parentheses.

	HV%	PHF
EB	6.2%	0.78
WB	3.9%	0.84
NB	1.8%	0.62
SB	0.0%	0.00
All	4.3%	0.82

Traffic Counts - Motorized Vehicles

Interval	NE L		OD CRE	EK RD	NE L		OD CRE bound			North	STORM Anbound		ŅE		Rolling			
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	Total	Hou
7:00 AM	0	0	12	4	0	0	8	0	0	1	0	1	0	0	0	0	26	31
7:05 AM	0	0	6	0	0	0	8	0	0	2	0	0	0	0	0	0	16	31
7:10 AM	0	0	6	2	0	1	12	0	0	1	0	0	0	0	0	0	22	32
7:15 AM	0	0	8	5	0	0	10	0	0	3	0	0	0	0	0	0	26	32
7:20 AM	0	0	4	0	0	0	21	0	0	3	0	1	0	0	0	0	29	31
7:25 AM	0	0	7	3	0	0	11	0	0	4	0	1	0	0	0	0	26	31
7:30 AM	0	0	8	1	0	1	17	0	0	6	0	0	0	0	0	0	33	31
7:35 AM	0	0	15	4	0	1	13	0	0	7	0	0	0	0	0	0	40	30
7:40 AM	0	0	3	0	0	1	10	0	0	10	0	0	0	0	0	0	24	29
7:45 AM	0	0	10	0	0	1	10	0	0	3	0	2	0	0	0	0	26	29
7:50 AM	0	0	9	1	0	0	10	0	0	2	0	0	0	0	0	0	22	28
7:55 AM	0	0	8	2	0	0	14	0	0	3	0	2	0	0	0	0	29	28
8:00 AM	0	0	7	0	0	2	8	0	0	4	0	0	0	0	0	0	21	28
8:05 AM	0	0	8	2	0	0	12	0	0	4	0	0	0	0	0	0	26	
8:10 AM	0	0	5	2	0	0	13	0	0	2	0	0	0	0	0	0	22	
8:15 AM	0	0	2	0	0	0	12	0	0	3	0	0	0	0	0	0	17	
8:20 AM	0	0	8	1	0	0	17	0	0	1	0	1	0	0	0	0	28	
8:25 AM	0	0	7	1	0	1	18	0	0	4	0	0	0	0	0	0	31	
8:30 AM	0	0	6	2	0	0	8	0	0	2	0	0	0	0	0	0	18	
8:35 AM	0	0	13	0	0	2	10	0	0	0	0	1	0	0	0	0	26	
8:40 AM	0	0	2	0	0	0	14	0	0	5	0	3	0	0	0	0	24	
8:45 AM	0	0	5	2	0	1	15	0	0	2	0	0	0	0	0	0	25	
8:50 AM	0	0	8	2	0	0	10	0	0	0	0	1	0	0	0	0	21	
8:55 AM	0	0	6	4	0	1	13	0	0	4	0	1	0	0	0	0	29	
Count Total	0	0	173	38	0	12	294	0	0	76	0	14	0	0	0	0	607	_
Peak Hour	0	0	93	20	0	7	148	0	0	50	0	6	0	0	0	0	324	

Traffic Counts - Heavy Vehicles, Bicycles on Road, and Pedestrians/Bicycles on Crosswalk

Interval		Hea	vy Vehicle	es		Interval		Bicycle	s on Road	dway		Interval	Ped	destrians/E	Bicycles on	Crosswa	lk
Start Time	EB	NB	WB	SB	Total	Start Time	EB	NB	WB	SB	Total	Start Time	EB	NB	WB	SB	Total
7:00 AM	1	0	0	0	1	7:00 AM	0	0	0	0	0	7:00 AM	0	1	0	0	1
7:05 AM	1	0	2	0	3	7:05 AM	0	0	0	0	0	7:05 AM	0	0	0	0	0
7:10 AM	0	0	0	0	0	7:10 AM	0	0	0	0	0	7:10 AM	0	0	0	0	0
7:15 AM	0	0	0	0	0	7:15 AM	0	0	0	0	0	7:15 AM	0	0	0	0	0
7:20 AM	1	0	1	0	2	7:20 AM	0	0	0	0	0	7:20 AM	0	0	0	0	0
7:25 AM	0	0	0	0	0	7:25 AM	0	0	0	0	0	7:25 AM	0	0	0	0	0
7:30 AM	1	0	1	0	2	7:30 AM	0	0	0	0	0	7:30 AM	0	1	0	0	1
7:35 AM	2	0	1	0	. 3	7:35 AM	0	0	0	0	0	7:35 AM	0	0	0	0	0
7:40 AM	0	0	0	0	0	7:40 AM	0	0	0	0	0	7:40 AM	0	1	0	0	1
7:45 AM	0	1	0	0	1	7:45 AM	0	0	0	0	0	7:45 AM	0	0	0	0	0
7:50 AM	2	0	0	0	2	7:50 AM	0	0	0	0	0	7:50 AM	0	0	0	0	0
7:55 AM	0	0	1	0	1	7:55 AM	0	0	0	0	0	7:55 AM	0	0	0	0	0
8:00 AM	1	0	1	0	2	8:00 AM	0	0	0	0	0	8:00 AM	0	0	0	0	0
8:05 AM	0	0	1	0	1	8:05 AM	0	0	0	0	0	8:05 AM	0	0	0	0	0
8:10 AM	1	0	1	0	2	8:10 AM	0	0	0	0	0	8:10 AM	0	0	0	0	0
8:15 AM	0	0	0	0	0	8:15 AM	0	0	0	0	0	8:15 AM	0	0	0	0	0
8:20 AM	1	0	0	0	1	8:20 AM	0	0	0	0	0	8:20 AM	0	0	0	0	0
8:25 AM	0	0	2	0	2	8:25 AM	0	0	0	0	0	8:25 AM	0	0	0	0	0
8:30 AM	0	0	0	0	0	8:30 AM	0	0	0	0	0	8:30 AM	0	0	0	0	0
8:35 AM	2	1	1	0	4	8:35 AM	1	0	0	0	1	8:35 AM	0	2	0	0	2
8:40 AM	0	0	1	0	1	8:40 AM	0	0	0	0	0	8:40 AM	0	0	0	0	0
8:45 AM	0	0	0	0	0	8:45 AM	0	0	0	0	0	8:45 AM	0	0	0	0	0
8:50 AM	1	0	0	0	1	8:50 AM	0	0	0	0	0	8:50 AM	0	0	0	0	0
8:55 AM	0	0	0	0	0	8:55 AM	0	0	0	0	0	8:55 AM	0	1	0	0	1
Count Total	14	2	13	0	29	Count Total	1	0	0	0	1	Count Total	0	6	0	0	6
Peak Hour	7	1	6	0	14	Peak Hour	0	0	0	0	0	Peak Hour	0	2	0	0	2



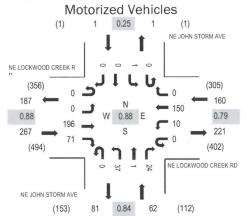
Location: 1 NE JOHN STORM AVE & NE LOCKWOOD CREEK RD PM

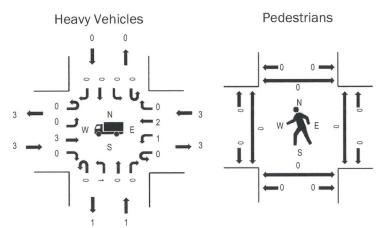
Date: Tuesday, July 27, 2021

Peak Hour: 04:25 PM - 05:25 PM

Peak 15-Minutes: 04:25 PM - 04:40 PM

Peak Hour





Note: Total study counts contained in parentheses.

	HV%	PHF
EB	1.1%	0.88
WB	1.9%	0.79
NB	1.6%	0.84
SB	0.0%	0.25
All	1.4%	0.88

Traffic Counts - Motorized Vehicles

Interval	NE L		OD CRE	EK RD (NE L		OD CRE bound	EK RD	NE		STORM Anbound	VE		South	STORM Anbound			Rollin
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	Total	Hou
4:00 PM	0	0	13	5	0	0	9	0	0	5	0	2	0	0	0	0	34	47
4:05 PM	0	0	11	4	0	1	16	0	0	2	0	2	0	0	0	0	36	47
4:10 PM	0	0	14	10	0	2	15	0	0	4	0	1	0	0	0	0	46	47
4:15 PM	0	0	13	5	0	0	7	0	0	4	0	1	0	0	0	0	30	46
4:20 PM	0	0	10	7	0	0	12	0	0	3	0	0	0	0	0	0	32	48
4:25 PM	0	0	12	9	0	1	11	0	0	5	0	4	0	0	0	0	42	49
4:30 PM	0	0	19	5	0	0	16	0	0	4	0	1	0	0	0	0	45	48
4:35 PM	0	0	18	6	0	3	18	0	0	3	0	4	0	0	0	0	52	47
4:40 PM	0	0	13	5	0	1	13	0	0	8	0	2	0	0	0	0	42	4!
4:45 PM	0	0	19	5	0	2	13	0	0	1	0	2	0	0	0	0	42	45
4:50 PM	0	0	16	7	0	0	8	0	0	3	0	1	0	0	0	0	35	4
4:55 PM	0	0	12	6	0	0	8	0	0	6	0	6	0	0	0	0	38	4
5:00 PM	0	0	17	5	0	2	10	0	0	0	0	0	0	0	0	0	34	4:
5:05 PM	0	0	12	5	0	1	18	0	0	1	0	2	0	0	0	0	39	
5:10 PM	0	0	18	6	0	0	8	0	0	2	1	1	0	0	0	0	36	
5:15 PM	0	0	23	6	0	0	9	0	0	3	0	1	0	1	0	0	43	
5:20 PM	0	0	17	6	0	0	18	0	0	1	0	0	0	0	0	0	42	
5:25 PM	0	0	16	3	0	0	14	0	0	3	0	1	0	0	0	0	37	
5:30 PM	0	0	14	8	0	1	9	0	0	1	0	1	0	0	0	0	34	
5:35 PM	0	0	10	3	0	2	14	0	0	5	0	2	0	0	0	0	36	
5:40 PM	0	0	15	6	0	1	16	0	0	1	0	1	0	0	0	0	40	
5:45 PM	0	0	21	4	0	0	8	0	0	2	0	3	0	0	0	0	38	
5:50 PM	0	0	14	3	0	2	9	0	0	0	0	3	0	0	0	0	31	
5:55 PM	0	0	13	5	0	0	7	0	0	3	0	0	0	0	0	0	28	
Count Total	0	0	360	134	0	19	286	0	0	70	1	41	0	1	0	0	912	`
Peak Hour	0	0	196	71	0	10	150	0	0	37	1	24	0	1	0	0	490)

Traffic Counts - Heavy Vehicles, Bicycles on Road, and Pedestrians/Bicycles on Crosswalk

Interval		Hea	avy Vehicle	es		Interval		Bicycle	es on Road	dway	χ.,	Interval	Pe	destrians/E	Bicycles on	Crosswa	lk
Start Time	EB	NB	WB	SB	Total	Start Time	EB	NB	WB	SB	Total	Start Time	EB	NB	WB	SB	Total
4:00 PM	0	0	0	0	0	4:00 PM	0	0	0	0	0	4:00 PM	0	0	0	0	0
4:05 PM	1	0	0	0	1	4:05 PM	0	0	0	0	0	4:05 PM	0	0	0	0	0
4:10 PM	0	0	0	0	0	4:10 PM	0	0	0	0	0	4:10 PM	0	0	0	0	0
4:15 PM	0	0	0	0	0	4:15 PM	0	0	0	0	0	4:15 PM	0	0	0	0	0
4:20 PM	0	0	0	0	0	4:20 PM	0	0	0	0	0	4:20 PM	0	0	0	0	0
4:25 PM	0	0	0	- 0	0	4:25 PM	0	0	0	0	0	4:25 PM	0	1.	0	0	1
4:30 PM	0	0	1	0	1	4:30 PM	0	0	0	0	0	4:30 PM	0	0	0	0	0
4:35 PM	1	0	0	0	1	4:35 PM	0	0	0	0	0	4:35 PM	0	0	0	0	0
4:40 PM	0	0	0	0	0	4:40 PM	0	0	0	0	0	4:40 PM	0	0	0	0	0
4:45 PM	0	0	1	0	1	4:45 PM	0	0	0	0	0	4:45 PM	0	0	0	0	0
4:50 PM	1	0	0	0	1	4:50 PM	0	0	0	0	0	4:50 PM	0	0	0	0	0
4:55 PM	0	0	0	0	0	4:55 PM	0	0	0	0	0	4:55 PM	0	0	0	0	0
5:00 PM	0	0	0	0	0	5:00 PM	0	0	0	0	0	5:00 PM	0	0	0	0	0
5:05 PM	0	1	0	0	1	5:05 PM	1	0	0	0	1	5:05 PM	0	0	0	0	0
5:10 PM	1	0	0	0	1	5:10 PM	0	0	0	0	0		0	0	0	0	0
5:15 PM	0	0	0	0	0	5:15 PM	0	0	0	0	0	5:15 PM	0	0	0	0	0
5:20 PM	0	0	1	0	1	5:20 PM	0	0	0	0	0	5:20 PM	0	0	0	0	0
5:25 PM	0	0	1	0	1	5:25 PM	0	0	0	0	0	5:25 PM	0	1	0	0	- 1
5:30 PM	0	0	0	0	0	5:30 PM	0	0	0	0	0	5:30 PM	0	0	0	0	0
5:35 PM	0	0	0	0	0	5:35 PM	0	0	0	0	C	5:35 PM	0	0	0	0	0
5:40 PM	0	0	0	0	0	5:40 PM	0	0	0	0	C	5:40 PM	0	0	0	0	0
5:45 PM	0	0	0	0	0	5:45 PM	0	0	0	0	C		0	0	0	0	0
5:50 PM	0	0	0	0	0	5:50 PM	0	0	0	0	C		0	0	0	0	0
5:55 PM	0	0	0	0	0	5:55 PM	0	0	0	0			0	0	0	0	0
Count Total	4	1	4	0	9	Count Total	1	0	0	0	1	1 Count Total	0	2	0	0	2
Peak Hour	3	1	3	0	7	Peak Hour	1	0	0	0	1	Peak Hour	0	1	0	0	1

Total Vehicle Summary



Clay Carney (503) 833-2740

E Ivy St & E 4th St

Thursday, September 13, 2018 7:00 AM to 9:00 AM

15-Minute Interval Summary

7:00 AM	to	9:00 A	М	•													
Interval Start			bound v St				bound v St				ound th St			Westl E 4t	ound h St		Interval
Time	L	T	R	Bikes	L	Т	R	Bikes	L	Т	R	Bikes	L	T	R	Bikes	Total
7:00 AM	0	0	0	0	0	0	33	0	8	21	2	1	0	45	0	0	109
7:15 AM	1	1	0	0	1	0	27	0	21	23	0	0	1	48	4	0	127
7:30 AM	0	0	0	0	0	0	36	0	23	20	0	0	0	55	2	0	136
7:45 AM	2	1	3	0	1	0	47	0	41	23	2	0	3	52	12	0	187
8:00 AM	13	0	7	0	8	3	69	0	51	36	9	0	4	53	17	0	270
8:15 AM	13	0	7	0	0	0	40	0	17	39	3	0	2	60	4	0	185
8:30 AM	3	0	2	0	1	0	23	0	10	20	2	0	0	32	2	0	95
8:45 AM	1	1	1	0	0	0	18	0	12	27	0	0	1	34	0	0	95
Total Survey	33	3	20	0	11	3	293	0	183	209	18	1	11	379	41	0	1,204

Peak Hour Summary 7:30 AM to 8:30 AM

Ву			bound y St				bound y St				ound th St				bound th St		Total
Approach	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes	
Volume	46	26	72	0	204	168	372	0	264	440	704	0	264	144	408	0	778
%HV		39.	1%			8.3	3%			6.4	4%			4.	2%		8.1%
PHF		0.	58			0.	64			0.	69			0.	89		0.72

al	H	Cross	swalk	
	North	South	East	West
3	1	8	0	1
6				
2	1			
	-			
	1			

15 0

Pedestrians

1 35

4 220

39.1% 0.58

로 높

264 In

Pedestrians
Crosswalk
North South East West

144 Out

HV 8.3% PHF 0.64

132

118 -

HV 6.4% PHF 0.69

Out 440

In 264

192

28

Out 26

Peak Hour Summary 7:30 AM to 8:30 AM

By Movement			bound y St				bound y St	7			ound th St			Westk E 4t	ound h St		Total
Movement	L	Т	R	Total	L	Т	R	Total	L	Т	R	Total	L	T	R	Total	
Volume	28	1	17	46	9	3	192	204	132	118	14	264	9	220	35	264	778
%HV	60.7%	0.0%	5.9%	39.1%	11.1%	0.0%	8.3%	8.3%	6.1%	7.6%	0.0%	6.4%	0.0%	3.6%	8.6%	4.2%	8.1%
PHF	0.54	0.25	0.61	0.58	0.28	0.25	0.70	0.64	0.65	0.76	0.39	0.69	0.56	0.92	0.51	0.89	0.72

Rolling Hour Summary 7:00 AM to 9:00 AM

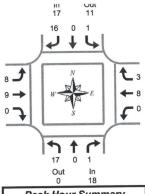
Interval Start			bound y St				bound ry St			Eastb E 4t	ound h St			Westb E 4t			Interval		Pedes Cross	trians swalk	
Time	L	T	R	Bikes	L	Т	R	Bikes	L	Т	R	Bikes	L	T	R	Bikes	Total	North	South	East	West
7:00 AM	3	2	3	0	2	0	143	0	93	87	4	1	4	200	18	0	559	0	10	0	1
7:15 AM	16	2	10	0	10	3	179	0	136	102	11	0	8	208	35	0	720	1	12	0	1
7:30 AM	28	1	17	0	9	3	192	0	132	118	14	0	9	220	35	0	778	1	8	0	1
7:45 AM	31	1	19	0	10	3	179	0	119	118	16	0	9	197	35	0	737	1	9	0	0
8:00 AM	30	1	17	0	9	3	150	0	90	122	14	0	7	179	23	0	645	1	5	0	0

Heavy Vehicle Summary



Clay Carney (503) 833-2740

Out 41 In 17



Peak Hour Summary 7:30 AM to 8:30 AM

E Ivy St & E 4th St

Thursday, September 13, 2018 7:00 AM to 9:00 AM

Heavy Vehicle 15-Minute Interval Summary

7:00 AM to 9:00 AM

Interval Start		North!	oound v St				bound y St			Eastb E 4t	ound h St			Westk E 4t	h St		Interval
Time	L	T	R	Total	L	Т	R	Total	L	Т	R	Total	L	Т	R	Total	Total
7:00 AM	0	0	0	0	0	0	1	1	1	8	0	9	0	1	0	1	11
7:15 AM	0	0	0	0	0	0	0	0	1	6	0	7	0	0	0	0	7
7:30 AM	0	0	0	0	0	0	0	0	0	1	0	1	0	1	0	1	2
		0	4	1	0	0	2	2	3	2	0	5	0	4	0	4	12
7:45 AM	0	-	0	12	1	0	14	15	4	3	0	7	0	2	2	4	38
8:00 AM	12	0	-	-	-	0	0	0	1	3	0	4	0	1	1	2	11
8:15 AM	5	0	0	5	0	-			1	2	0	3	0	2	0	2	5
8:30 AM	0	0	0	0	0	0	0	0	-		0	3		-	0		6
8:45 AM	0	0	0	0	0	0	1	1	0	1	0	1	0	4	U	4	0
Total Survey	17	0	1	18	1	0	18	19	11	26	0	37	0	15	3	18	92

Heavy Vehicle Peak Hour Summary

7:30 AM to 8:30 AM

Ву			bound vy St			bound vy St			bound th St			bound th St	Total
Approach	In	Out	Total										
Volume	18	0	18	17	11	28	17	41	58	11	11	22	63
PHF	0.25	0	10	0.25			0.25			0.28			0.26

Ву			bound v St			South E lv	bound y St				ound h St			Westl E 4t	oound h St		Total
Movement	1	Т	R	Total	1	T	R	Total	L	Т	R	Total	L	Т	R	Total	
Volume	17	0	1	18	1	0	16	17	8	9	0	17	0	8	3	11	63
PHF	0.25	0.00	0.25	0.25	0.25	0.00	0.25	0.25	0.25	0.15	0.00	0.25	0.00	0.29	0.25	0.28	0.26

Heavy Vehicle Rolling Hour Summary 7:00 AM to 9:00 AM

1:00 AIVI	10 :	J.UU A	IVI														
Interval Start			bound v St				bound y St				ound h St			Westk E 4t	h St		Interval
Time	L	Т	R	Total	L	Т	R	Total	L	Т	R	Total	L	T	R	Total	Total
7:00 AM	0	0	1	1	0	0	3	3	5	17	0	22	0	6	0	6	32
	40	0	4	13	1	0	16	17	8	12	0	20	0	7	2	9	59
7:15 AM	12	U	1		1	0	-	47	0	0	0	17	0	R	3	11	63
7:30 AM	17	0	1	18	1	0	16	17	8	9	U	17	0	0	3	40	
7:45 AM	17	0	1	18	1	0	16	17	9	10	0	19	0	9	3	12	66
8:00 AM	47	0	0	17	1	0	15	16	6	9	0	15	0	9	3	12	60

Peak Hour Summary All Traffic Data Clay Carney (503) 833-2740 E Ivy St & E 4th St 7:30 AM to 8:30 AM Thursday, September 13, 2018 E lvy St **Bikes** 0 204 168 192 3 9 1 4 Peds 1 E 4th St Bikes 0 35 440 220 264 132 264 144 118 14 3 Bikes 0 E 4th St Peds 8 7 28 17 26 46 **Bikes** HV% **PHF** Volume **Approach EB** 0.69 6.4% 264 **WB** 0.89 4.2% 264 0.58 39.1% 46 NB SB 0.64 8.3% 204 Intersection 0.72 8.1% 778 Count Period: 7:00 AM to 9:00 AM

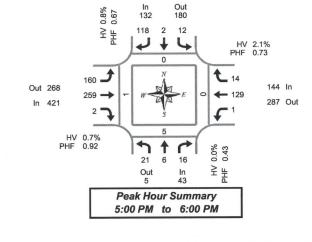
Total Vehicle Summary



E Ivy St & E 4th St

Thursday, September 13, 2018 4:00 PM to 6:00 PM

15-Minute Interval Summary 4:00 PM to 6:00 PM



Interval Start		North	bound v St				bound v St			Eastb E 4t				Westb E 4t			Interval		Pedes		
Time	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	L	Т	R	Bikes	Total	North	South	East	West
4:00 PM	2	0	1	0	2	0	31	0	24	46	2	0	0	39	1	0	148	0	21	0	0
4:15 PM	5	0	3	0	2	1	20	0	39	64	4	0	0	46	1	0	185	0	20	0	0
4:30 PM	6	1	4	0	0	0	17	0	39	57	1	0	0	44	3	0	172	0	32	0	0
4:45 PM	0	0	0	0	3	1	18	0	49	76	2	0	1	45	5	0	200	0	11	0	0
5:00 PM	1	0	0	0	5	1	22	0	46	59	0	0	0	28	7	0	169	0	1	0	0
5:15 PM	1	1	0	0	1	0	22	0	30	61	. 0	0	0	46	3	0	165	0	0	0	0
5:30 PM	14	4	7	0	0	0	32	0	45	68	2	0	0	30	3	0	205	0	1	0	0
5:45 PM	5	1	9	0	6	1	42	0	39	71	0	0	1	25	1	0	201	0	3	0	1
Total Survey	34	7	24	0	19	4	204	0	311	502	11	0	2	303	24	0	1,445	0	89	0	1

Peak Hour Summary 5:00 PM to 6:00 PM

Ву			bound ry St				bound y St				oound th St				bound th St		Total
Approach	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes	
Volume	43	5	48	0	132	180	312	0	421	268	689	0	144	287	431	0	740
%HV		0.1	0%			0.8	3%			0.	7%		2.1%				0.9%
PHF		0.	43			0.	67			0.	92			0.	73		0.90

	Pedes	trians	
	Cross	swalk	
North	South	East	West
0	5	0	1

Ву			bound v St				bound y St			Easth E 4t	ound h St			Westl E 4t	oound h St		Total
Movement	lovement L	Т	R	Total	L	Т	R	Total	L	T	R	Total	L	T	R	Total	
Volume	21	6	16	43	12	2	118	132	160	259	2	421	1	129	14	144	740
%HV	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.8%	0.8%	0.6%	0.8%	0.0%	0.7%	0.0%	2.3%	0.0%	2.1%	0.9%
PHF	0.38	0.38	0.44	0.43	0.50	0.50	0.70	0.67	0.87	0.91	0.25	0.92	0.25	0.70	0.50	0.73	0.90

Rolling Hour Summary 4:00 PM to 6:00 PM

Interval Start			bound y St				bound y St			Eastb E 4t				Westb E 4t			Interval
Time	L	T	R	Bikes	L	Т	R	Bikes	L	Т	R	Bikes	L	T	R	Bikes	Total
4:00 PM	13	1	8	0	7	2	86	0	151	243	9	0	1	174	10	0	705
4:15 PM	12	1	7	0	10	3	77	0	173	256	7	0	1	163	16	0	726
4:30 PM	8	2	4	0	9	2	79	0	164	253	3	0	1	163	18	0	706
4:45 PM	16	5	7	0	9	2	94	0	170	264	4	0	1	149	18	0	739
5:00 PM	21	6	16	0	12	2	118	0	160	259	2	0	1	129	14	0	740

	Cross		
North	South	East	West
0	84	0	0
0	64	0	0
0	44	0	0
0	13	0	0
0	5	0	1

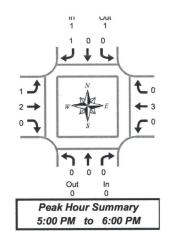
Heavy Vehicle Summary



Clay Carney (503) 833-2740

E Ivy St & E 4th St

Thursday, September 13, 2018 4:00 PM to 6:00 PM



Out 4

In 3

Heavy Vehicle 15-Minute Interval Summary

4:00 PM to 6:00 PM

Interval Start			bound ry St		Southbound E Ivy St						oound th St			Westl E 4t	oound h St		Interval
Time	L	Т	R	Total	L	Т	R	Total	L	Т	R	Total	L	T	R	Total	Total
4:00 PM	0	0	0	0	0	0	1	1	0	2	0	2	0	4	0	4	7
4:15 PM	0	0	0	0	0	0	1	1	0	2	0	2	0	2	0	2	5
4:30 PM	0	0	0	0	0	0	1	1	0	1	0	1	0	1	0	1	3
4:45 PM	0	0	0	0	0	0	2	2	0	2	0	2	0	1	0	1	5
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0	1	0	1	0	2	0	2	3
5:30 PM	0	0	0	0	0	0	0	0	0	1	0	1	0	1	0	1	2
5:45 PM	0	0	0	0	0	0	1	1	1	0	0	1	0	0	0	0	2
Total Survey	0	0	0	0	0	0	6	6	1	9	0	10	0	11	0	11	27

Heavy Vehicle Peak Hour Summary 5:00 PM to 6:00 PM

Ву			bound ry St			nbound vy St			bound th St			bound th St	Total
Approach	In	Out	Total	In	Out	Total	In	Out	Total	In	Out	Total	
Volume	0	0	0	1	1	2	3	4	7	3	2	5	7
PHF	0.00	0.00					0.15			0.11			0.12

Ву			bound y St				bound y St				ound h St				oound th St		Total
Movement	L	Т	R	Total	L	Т	R	Total	L	Т	R	Total	L	T	R	Total	
Volume	0	0	0	0	0	0	1	1	1	2	0	3	0	3	0	3	7
PHF	0.00	0.00	0.00	0.00	0.00	0.00	0.06	0.06	0.25	0.10	0.00	0.15	0.00	0.11	0.00	0.11	0.12

Heavy Vehicle Rolling Hour Summary 4:00 PM to 6:00 PM

Interval Start			bound y St				bound y St				oound th St			Westl E 4t	ound h St		Interval
Time	L	Т	R	Total	L	Т	R	Total	L	Т	R	Total	L	Ť	R	Total	Total
4:00 PM	0	0	0	0	0	0	5	5	0	7	0	7	0	8	0	8	20
4:15 PM	0	0	0	0	0	0	4	4	0	5	0	5	0	4	0	4	13
4:30 PM	0	0	0	0	0	0	3	3	0	4	0	4	0	4	0	4	11
4:45 PM	0	0	0	0	0	0	2	2	0	4	0	4	0	4	0	4	10
5:00 PM	0	0	0	0	0	0	1	1	1	2	0	3	0	3	0	3	7

Peak Hour Summary All Traffic Data Clay Carney (503) 833-2740 E Ivy St & E 4th St 5:00 PM to 6:00 PM Thursday, September 13, 2018 E lvy St **Bikes** 0 132 180 118 2 12 K 3 Peds 0 E 4th St Bikes 0 14 129 144 268 1 0 160 287 421 259 -2 Bikes 0 E 4th St Peds 5 1 1 7 21 6 16 43 **Bikes** HV% **Approach** PHF Volume EB 0.92 0.7% 421 WB 0.73 2.1% 144 43 0.43 0.0% NB SB 0.67 0.8% 132 Intersection 0.90 0.9% 740 Count Period: 4:00 PM to 6:00 PM

INTERSECTION TURN MOVEMENT SURVEY ASPEN AVENUE & E 4TH STREET

DATE OF COUNT: 2/15/2022, 07:00-09:00

DAY OF WEEK:

TUE.

WEATHER:

CLOUDY

KAK COUNTER:

Time Period From – To	FRON	I NO	RTH	FRO	OM EAS	ST	FRO	M SOI	JTH	FRO	M WE	ST 7	TOTAL
110111 10	L	T	R	L	T	R	L	T	R	L	T	R	
07:00-07:05	1	0	15	0	33	0	0	0	0	2	12	0	63
07:05-07:10	1	0	10	0	23	1	0	0	0	1	10	0	46
07:10-07:15	0	0	9	0	30	0	0	0	0	3	19	0	61
07:15-07:20	0	0	14	0	24	0	0	0	0	2	15	0	55
07:20-07:25	2	0	12	0	20	1	0	0	0	2	14	0	51
07:25-07:30	1	0	14	0	41	1	0	0	0	1	23	0	81
07:30-07:35	0	0	12	0	21	1	0	0	0	0	18	0	52
07:35-07:40	4	0	10	0	16	0	0	0	0	8	17	0	55
07:40-07:45	8	0	14	0	35	0	0	0	0	3	29	0	89
07:45-07:50	6	0	9	0	33	100	0	0	0	4	42	0	95
07:50-07:55	11	0	4	0	36	- Samuel	0	0	0	2	41	0	95
07:55-08:00	7	0	14	0	35	0	0	0	0	4	42	0	102
08:00-08:05	3	0	7	0	55	0	0	0	0	6	27	0	98
08:05-08:10	2	0	6	0	61	6	0	0	0	3	14	0	92
08:10-08:15	100	0	9	0	43	4	0	0	0	4	13	0	74
08:15-08:20	2	0	13	0	29	1	0	0	0	8	17	0	70
08:20-08:25	The same of	0	8	0	20	1	0	0	0	7	13	0	50
08:25-08:30	0	0	13	0	25	1	0	0	0	3	12	0	54
08:30-08:35	1	0	13	0	19	1	0	0	0	2	13	0	49
08:35-08:40	0	0	5	0	13	0	0	0	0	7	8	0	33
08:40-08:45	2	0	7	0	17	1	0	0	0	3	9	0	39
08:45-08:50	0	0	10	0	15	0	0	0	0	4	10	0	39
08:50-08:55	1	0	9	0	20	2	0	0	0	2	7	0	41
08:55-09:00	0	0	8	0	16	0	0	0	0	3	11	0	38
Peak Hour To	tal 47	0	124	0	425	16	0	0	0	45	297	0	954
% Trucks	4	0	0	0	0	6	0	0	0	7	4	0	
Peds	0	0	0	0	0	0	0	0	0	0	0	0	
Bikes	0	0	0	0	0	0	0	0	0	0	0	0	

PEAK HOUR: 07:20-08:20

INTERSECTION TURN MOVEMENT SURVEY ASPEN AVENUE & E 4^{TH} STREET

DATE OF COUNT: 2/15/2022, 16:00-18:00

DAY OF WEEK:

TUE.

WEATHER:

CLOUDY

COUNTER:

KAK

Time Period From – To	FROM	M NO	RTH	FRO	OM EAS	ST	FRO	M SOU	JTH	FRO	M WE	ST	TOTAL
F10III — 10	L	T	R	L	T	R	L	T	R	L	T	R	
	L	1	10	L	1	14	L	1		2	•		
16:00-16:05	1	0	5	0	26	0	0	0	0	14	29	0	75
16:05-16:10	3	0	6	0	28	3	0	0	0	13	35	0	88
16:15-16:20	2	0	10	0	13	0	0	0	0	13	39	0	77
16:10-16:15	2	0	7	0	14	0	0	0	0	14	36	0	73
16:20-16:25	1	0	4	0	19	1	0	0	0	9	31	0	65
16:25-16:30	1	0	6	0	20	0	0	0	0	9	26	0	62
16:30-16:35	0	0	1	0	11	1	0	0	0	8	40	0	61
16:35-16:40	0	0	5	0	21	0	0	0	0	14	31	0	71
16:40-16:45	0	0	4	0	17	3	0	0	0	12	36	0	72
16:45-16:50	0	0	7	0	15	1	0	0	0	15	46	0	84
16:50-16:55	2	0	5	0	16	0	0	0	0	18	31	0	72
16:55-17:00	2	0	8	0	21	3	0	0	0	14	26	0	74
17:00-17:05	2	0	8	0	25	0	0	0	0	17	33	0	85
17:05-17:10	0	0	5	0	21	0	0	0	0	9	38	0	73
17:10-17:15	3	0	11	0	16	0	0	0	0	13	41	0	84
17:15-17:20	1	0	7	0	30	1	0	0	0	13	30	0	82
17:20-17:25	1	0	7	0	13	0	0	0	0	23	40	0	84
17:25-17:30	0	0	8	0	18	0	0	0	0	8	47	0	81
17:30-17:35	0	0	12	0	15	0	0	0	0	12	28	0	67
17:35-17:40	0	0	5	0	22	3	0	0	0	17	29	0	76
17:40-17:45	1	0	3	0	15	0	0	0	0	10	30	0	59
17:45-17:50	0	0	4	0	14	1	0	0	0	9	27	0	55
17:50-17:55	1	0	2	0	13	0	0	0	0	11	28	0	55
17:55-18:00	0	0	4	0	12	1	0	0	0	10	31	0	58
Peak Hour Tot	tal 11	0	87	0	229	11	0	0	0	171	425	0	934
% Trucks	0	0	0	0	1	0	0	0	0	0	0	0	
Peds	0	2	0	0	0	0	0	2	0	0	5	0	
Bikes	0	0	0	0	0	0	0	0	0	0	0	0	

PEAK HOUR: 16:40-17:40

ROUNDABOUT MOVEMENT SURVEY PACIFIC AVENUE & W 4TH STREET

DATE OF COUNT: 2/17/2022, 07:00-09:00

DAY OF WEEK:

THUR.

WEATHER:

CLOUDY

COUNTER:

KAK

	FROM	1 NOR	TH	FRO	M EA	ST	FRO	M SOU	TH	FRO	OM W	EST 7	TOTAL
From – To		-	D		Œ	D		Œ	n	т	т	D	
	L	T	R	L	T	R	L	T	R	L	T	R	
07:00-07:05	0	12	0	37	0	0	0	2	43	0	0	0	94
07:05-07:10	0	16	0	26	0	0	0	5	48	0	0	0	95
07:10-07:15	0	17	0	43	0	2	0	7	52	0	0	0	121
07:15-07:20	1	13	0	34	0	1	0	3	48	0	0	0	100
07:20-07:25	0	14	0	35	0	1	0	4	39	0	0	0	93
07:25-07:30	1	15	0	35	0	1	0	7	34	0	0	0	93
07:30-07:35	0	16	0	43	0	1	0	5	48	0	0	0	113
07:35-07:40	1	15	0	38	0	4	0	5	45	0	0	0	108
07:40-07:45	0	10	0	40	0	1	0	8	48	0	0	0	107
07:45-07:50	6	10	0	29	0	2	0	9	61	0	0	0	117
07:50-07:55	10	14	0	31	0	1	0	6	47	0	0	0	109
07:55-08:00	12	17	0	20	0	4	0	7	38	0	0	0	98
08:00-08:05	8	15	0	34	0	10	0	4	48	0	0	0	119
08:05-08:10	3	22	0	55	0	2	0	4	47	0	0	0	133
08:10-08:15	3	22	0	44	0	8	0	7	51	0	0	0	135
08:15-08:20	0	19	0	66	0	5	0	3	42	0	0	0	135
08:20-08:25	2	12	0	46	0	9	0	4	62	0	0	0	135
08:25-08:30	1	7	0	27	0	5	0	5	55	0	0	0	100
08:30-08:35	3	11	0	32	0	0	0	8	40	0	0	0	94
08:35-08:40	2	8	0	24	0	3	0	7	46	0	0	0	90
08:40-08:45	1	9	0	31	0	1	0	4	39	0	0	0	85
08:45-08:50	2	10	0	26	0	2	0	5	46	0	0	0	91
08:50-08:55	2	9	0	25	0	0	0	4	38	0	0	0	78
08:55-09:00	1	11	0	31	0	3	0	6	41	0	0	0	93
Peak Hour Tota	1 46	179	0	473	0	52	0	67	592	0	0	0	1409
% Trucks	9	1	0	0	0	4	0	3	4	0	0	0	
Peds	0	1	0	0	1	0	0	0	0	0	1	0	
Bikes	0	0	0	0	0	0	0	0	0	0	0	0	

PEAK HOUR: 07:30-08:30

ROUNDABOUT TURN MOVEMENT SURVEY PACIFIC AVENUE & W 4TH STREET

DATE OF COUNT: 2/16/2022, 16:00-18:00

DAY OF WEEK:

WED.

WEATHER:

CLOUDY

COUNTER:

KAK

From – To L T R L T R L T R L T R 16:00-16:05 2 15 0 29 0 8 0 17 41 0 0 0 112 16:05-16:10 1 19 0 24 0 8 0 9 47 0 0 0 108
16:00-16:05 2 15 0 29 0 8 0 17 41 0 0 0 112
10.00-10.05
1000
16:05-16:10 1 19 0 24 0 8 0 9 47 0 0 0 108
16:15-16:20 2 14 0 22 0 2 0 13 50 0 0 103
16:10-16:15 1 9 0 29 0 5 0 13 49 0 0 106
16:20-16:25
16:25-16:30 0 6 0 19 0 3 0 16 35 0 0 0 79
16:30-16:35 0 7 0 27 0 3 0 16 48 0 0 0 101
16:35-16:40 0 12 0 30 0 2 0 6 45 0 0 0 95
16:40-16:45 3 8 0 32 0 2 0 13 45 0 0 0 103
16:45-16:50 3 10 0 23 0 1 0 14 56 0 0 0 107
16:50-16:55 0 11 0 18 0 1 0 19 49 0 0 0 98
16:55-17:00 2 9 0 28 0 4 0 18 38 0 0 0 99
17:00-17:05 1 11 0 24 0 5 0 16 49 0 0 0 106
17:05-17:10 3 16 0 29 0 4 0 11 43 0 0 0 106
17:10-17:15 2 16 0 26 0 4 0 12 52 0 0 0 112
17:15-17:20 1 11 0 38 0 3 0 18 42 0 0 0 113
17:20-17:25 0 7 0 21 0 1 0 18 63 0 0 0 110
17:25-17:30 1 11 0 31 0 1 0 8 54 0 0 0 106
17:30-17:35 0 9 0 17 0 2 0 14 40 0 0 0 82
17:35-17:40
17:40-17:45
17:45-17:50 2 7 0 19 0 3 0 11 34 0 0 0 76
17:50-17:55 1 9 0 21 0 2 0 9 38 0 0 0 80
17:55-18:00 0 10 0 30 0 4 0 9 41 0 0 84
Peak Hour Total 16 129 0 327 0 31 0 169 584 0 0 0 1256
% Trucks 0 1 0 1 0 0 0 0 0 0 0
Peds 0 0 0 0 13 0 0 0 0 10 0
Bikes 0 0 0 0 0 0 0 0 0 0 0 0

PEAK HOUR: 16:30-17:30

Total Vehicle Summary



Clay Carney (503) 833-2740

NW Timmen Rd & NW La Center Rd

Thursday, May 09, 2019 7:00 AM to 9:00 AM

15-Minut 7:00 AM		rval Summ 9:00 AM	ary										
Interval Start		Northbound NW Timmen Ro	1	Southbound NW Timmen Ro	1	N	Eastb W La C		d	١	Westb W La C	d	Interva
Time	, L	R	Bikes		Bikes		T	R	Bikes	L	T	Bikes	Total
7:00 AM	3	7	0		0		44	2	0	13	135	0	204
7:15 AM	3	10	0		0		53	2	0	11	148	0	227
7:30 AM	3	8	0		0		67	1	0	22	130	0	231
7:45 AM	2	15	0		0		79	3	0	13	129	0	241
8:00 AM	2	8	0		0		59	2	0	9	150	0	230
8:15 AM	2	5	0		0		46	4	0	14	134	0	205
8:30 AM	5	9	0		0		59	2	0	17	111	0	203
8:45 AM	3	7	0		0		45	3	0	13	100	0	171
Total	23	69	0		0		452	19	0	112	1,037	0	1,712

		urians	
	Cross	swalk	
North	South	East	West
0	0	1	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	1	0

HV 4.4% PHF 0.96

← 557

3.9%

로 품

612 In

299 Out

00.0

HH H

258 -

10

Out 63

Peak Hour Summary 7:15 AM to 8:15 AM

HV 8.6% PHF 0.81

Out 567

In 266

Peak Hour Summary 7:15 AM to 8:15 AM

Ву		North NW Tim	bound nmen Rd			South NW Tim	bound nmen Rd		1	Eastb NW La C	oound Center R	d	1	Westl W La C	bound Center R	d	Total
Approach	In	Out	Total	Bikes	In	n Out Total Bikes		In	Out	Total	Bikes	In	Out	Total	Bikes		
Volume	51	63	114	0	0	0	0	0	266	567	833	0	612	299	911	0	929
%HV			9%			0.0	0%			8.6	6%			4.4	4%		5.6%
PHF			75		0.00					0.	81			0.	96		0.96

	Pedes		
North	South	East	West
0	0	0	0

Ву	Northbound NW Timmen Rd					South NW Tim	bound men R		1	Easth NW La C	oound Center R	d	N	Westh W La C		-	Total
Movement	L		R	Total				Total		Т	R	Total	L	T		Total	
Volume	10		41	51				0		258	8	266	55	557		612	929
%HV	10.0%	NA	2 4%	3.9%	NA	NA	NA	0.0%	NA	8.1%	25.0%	8.6%	0.0%	4.8%	NA	4.4%	5.6%
PHF	0.83	147		0.75				0.00		0.82	0.67	0.81	0.63	0.93		0.96	0.96

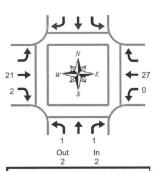
Rolling Hour Summary 7:00 AM to 9:00 AM

Interval Start		orthbound Timmen Ro	Timmen Rd NW Timmen Rd				ound enter R	ld	1	Westboo NW La Cen		Interval		Pedes	swalk	
Time	L	R	Bikes		Bikes	T	R	Bikes	L	T	Bikes	Total	North	South	East	West
7:00 AM	11	40	0		0	243	8	0	59	542	0	903	0	0	1	0
7:15 AM	10	41	0		0	258	8	0	55	557	0	929	0	0	0	0
7:30 AM	9	36	0		0	251	10	0	58	543	0	907	0	0	0	0
7:45 AM	11	37	0		0	243	11	0	53	524	0	879	0	0	0	0
8:00 AM	12	29	0		0	209	11	0	53	495	0	809	0	0	0	0

Heavy Vehicle Summary



Clay Carney (503) 833-2740 Out 28 In 23



Peak Hour Summary 7:15 AM to 8:15 AM

NW Timmen Rd & NW La Center Rd

Thursday, May 09, 2019 7:00 AM to 9:00 AM

Heavy Vehicle 15-Minute Interval Summary 7:00 AM to 9:00 AM

Interval Start		Northbound W Timmen Ro	3	Southb NW Timr		Eastb NW La C	ound Center F	Rd	1	Westl NW La C	bound Center Rd	Interval
Time	L	R	Total		Total	Т	R	Total	L	Т	Tota	Total
7:00 AM	0	0	0		0	11	2	13	1	6	7	20
7:15 AM	0	0	0		0	7	0	7	0	2	2	9
7:30 AM	0	1	1		0	4	0	4	0	3	3	8
7:45 AM	0	0	0		0	4	1	5	0	1	1	6
8:00 AM	1	0	1		0	6	1	7	0	21	21	29
8:15 AM	1	0	1		0	5	0	5	1	7	8	14
8:30 AM	4	1	5		0	5	0	5	0	3	3	13
8:45 AM	0	0	0		0	2	0	2	1	4	5	7
Total Survey	6	2	8		0	44	4	48	3	47	50	106

Heavy Vehicle Peak Hour Summary

7:15 AM to 8:15 AM

Ву			bound nmen Rd			bound nmen Rd	1		oound Center Rd			bound Center Rd	Total
Approach	In	Out	Total	In	Out	Total	In	Out	Total	In	Out	Total	
Volume	2	2	4	0	0	0	23	28	51	27	22	49	52
PHF	0.07	0.0					0.24			0.21			0.23

Ву		North NW Tim	bound men Rd		South NW Tim	bound men Rd		١		ound enter R	d	1		bound Center Ro	t t	Total
Movement	L		R	Total			Total		Т	R	Total	L	Т		Total	
Volume	1		1	2			0		21	2	23	-0	27		27	52
PHF	0.04		0.25	0.07			0.00		0.24	0.25	0.24	0.00	0.22		0.21	0.23

Heavy Vehicle Rolling Hour Summary 7:00 AM to 9:00 AM

Interval Start		Northbound V Timmen Ro	4	Southbo NW Timm		Eastl NW La 0	cound Center F	Rd		Westh NW La C	oound enter Rd	Interval
Time	L	R	Total		Total	T	R	Total	L	T	Total	Total
7:00 AM	0	1	1		0	26	3	29	1	12	13	43
7:15 AM	1	1	2		0	21	2	23	0	27	27	52
7:30 AM	2	1	3		0	19	2	21	1	32	33	57
7:45 AM	6	1	7		0	20	2	22	1	32	33	62
8:00 AM	6	1	7		0	18	1	19	2	35	37	63

Peak Hour Summary

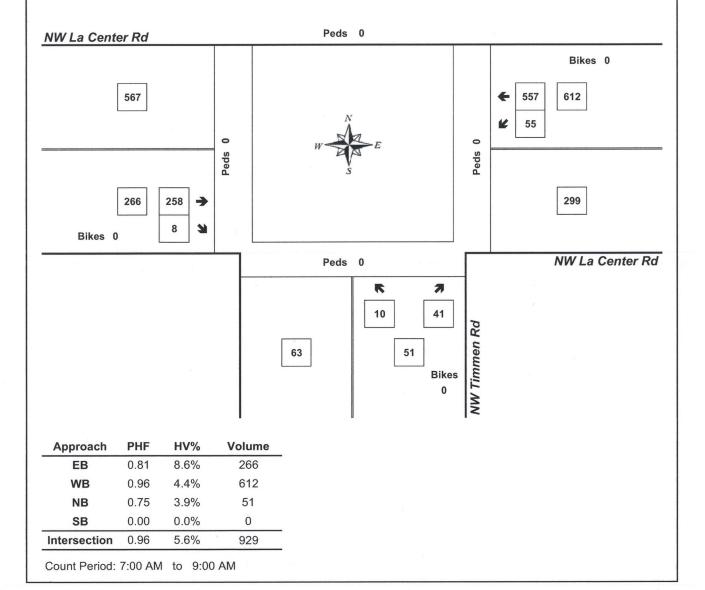


Clay Carney (503) 833-2740

NW Timmen Rd & NW La Center Rd

7:15 AM to 8:15 AM Thursday, May 09, 2019

Bikes 0



Total Vehicle Summary

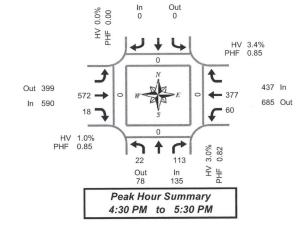


Clay Carney (503) 833-2740

NW Timmen Rd & NW La Center Rd

Wednesday, May 08, 2019 4:00 PM to 6:00 PM

15-Minute Interval Summary 4:00 PM to 6:00 PM



Interval		Northbou	nd			South	ound		Eastb	ound			West	oound			Pedes	trians	
Start		NW Timmer	Rd		N	W Tim	men Rd	N	W La C	enter R	d	1	NW La C	enter Rd	Interval		Cros	swalk	
Time	L	F	3	Bikes			Bikes		Т	R	Bikes	L	T	Bikes	Total	North	South	East	West
4:00 PM	4	2	1	0			0		133	6	0	11	102	0	277	0	0	0	0
4:15 PM	2	3	6	0			0		138	8	0	9	97	0	290	0	0	0	0
4:30 PM	8	3	3	0			0		139	5	0	11	84	0	280	0	0	- 0	0
4:45 PM	4	2	7	0			0		125	7	0	11	98	0	272	0	0	0	0
5:00 PM	3	2	3	0			0		138	3	0	18	111	0	296	0	0	0	0
5:15 PM	7	3	0	0			0		170	3	0	20	84	0	314	0	0	0	0
5:30 PM	5	1	5	0			0		167	3	0	4	78	0	272	0	0	0	0
5:45 PM	2	2	5	1			0		126	2	0	11	58	0	224	0	0	0	0
Total Survey	35	2	10	1			0		1,136	37	0	95	712	0	2,225	0	0	0	0

Peak Hour Summary 4:30 PM to 5:30 PM

Ву			bound nmen Rd			South NW Tim	bound nmen Rd		1		ound Center R	d	1		bound Center R	d	Total
Approach	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes	
Volume	135	78	213	0	0	0	0	0	590	399	989	0	437	685	1,122	0	1,162
%HV		3.0	0%			0.0	0%			1.0	0%			3.	4%		2.2%
PHF		0.	82			0.	00			0.	85			0.	.85		0.93

	Pedes	trians	
	Cross	swalk	
North	South	East	West
0	0	0	0

Ву		North NW Tim	bound men Ro	d		South NW Tim	bound men R		1	Eastb NW La C	ound Center R	ld.	1	Westh W La C		Rd	Total
Movement	L		R	Total				Total		Т	R	Total	L	T		Total	
Volume	22		113	135				0		572	18	590	60	377		437	1,162
%HV	9.1%	NA	1.8%	3.0%	NA	NA	NA	0.0%	NA	1.0%	0.0%	1.0%	0.0%	4.0%	NA	3.4%	2.2%
PHF	0.69		0.86	0.82				0.00		0.84	0.64	0.85	0.75	0.85		0.85	0.93

Rolling Hour Summary 4:00 PM to 6:00 PM

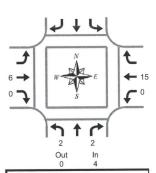
Interval Start	1	Northbound	4	Southboo NW Timme		Eastb NW La C		ld.	1	Westboo NW La Cen		Interval		Pedes Cross	trians swalk	
Time	L	R	Bikes		Bikes	T	R	Bikes	L	T	Bikes	Total	North	South	East	West
4:00 PM	18	117	0		0	535	26	0	42	381	0	1,119	0	0	0	0
4:15 PM	17	119	0		0	540	23	0	49	390	0	1,138	0	0	0	0
4:30 PM	22	113	0		0	572	18	0	60	377	0	1,162	0	0	0	0
4:45 PM	19	95	0		0	600	16	0	53	371	0	1,154	0	0	0	0
5:00 PM	17	93	1		0	601	11	0	53	331	0	1,106	0	0	0	0

Heavy Vehicle Summary



Clay Carney (503) 833-2740

Out 17 In 6



Peak Hour Summary 4:30 PM to 5:30 PM

NW Timmen Rd & NW La Center Rd

Wednesday, May 08, 2019 4:00 PM to 6:00 PM

Heavy Vehicle 15-Minute Interval Summary 4:00 PM to 6:00 PM

Interval Start		orthbound Timmen R		Southbound NW Timmen Rd		Eastb NW La C		ld	1	Westk NW La C	oound enter Rd	Interval
Time	L	R	Total		Total	T	R	Total	L	Т	Total	Total
4:00 PM	1	0	1		0	4	1	5	0	4	4	10
4:15 PM	0	0	0		0	4	0	4	0	6	6	10
4:30 PM	1	1	2		0	4	0	4	0	4	4	10
4:45 PM	0	0	0		0	2	0	2	0	3	3	5
5:00 PM	0	1	1		0	0	0	0	0	6	6	7
5:15 PM	1	0	1		0	0	0	0	0	2	2	3
5:30 PM	1	0	1		0	1	0	1	0	2	2	4
5:45 PM	0	0	0		0	1	1	2	0	1	1	3
Total Survey	4	2	6		0	16	2	18	0	28	28	52

Heavy Vehicle Peak Hour Summary 4:30 PM to 5:30 PM

Ву			bound nmen Rd			bound nmen Rd			bound Center Rd	1		bound Center Rd	Total
Approach	In	Out	Total	In	Out	Total	In	Out	Total	In	Out	Total	
Volume	4	0	4	0	0	0	6	17	23	15	8	23	25
PHF	0.33			0.00			0.12			0.27			0.21

Ву		 bound men Rd		 bound nmen Rd	East NW La	bound Center R	ld	1	Westh W La C	enter Rd	Total
Movement	L	R	Total	Total	T	R	Total	L	T	Total	
Volume	2	2	4	0	6	0	6	0	15	15	25
PHF	0.25	0.25	0.33	0.00	0.13	0.00	0.12	0.00	0.27	0.27	0.21

Heavy Vehicle Rolling Hour Summary 4:00 PM to 6:00 PM

Interval Start		orthbound Timmen R	-	Southbo NW Timme		Easth NW La C	oound Center R	d	1	Westbou NW La Cent		Interval
Time	L	R	Total		Total	T	R	Total	L	T	Total	Total
4:00 PM	2	1	3		0	14	1	15	0	17	17	35
4:15 PM	1	2	3		0	10	0	10	0	19	19	32
4:30 PM	2	2	4		0	6	0	6	0	15	15	25
4:45 PM	2	1	3		0	3	0	3	0	13	13	19
5:00 PM	2	1	3		0	2	1	3	0	11	11	17

Peak Hour Summary

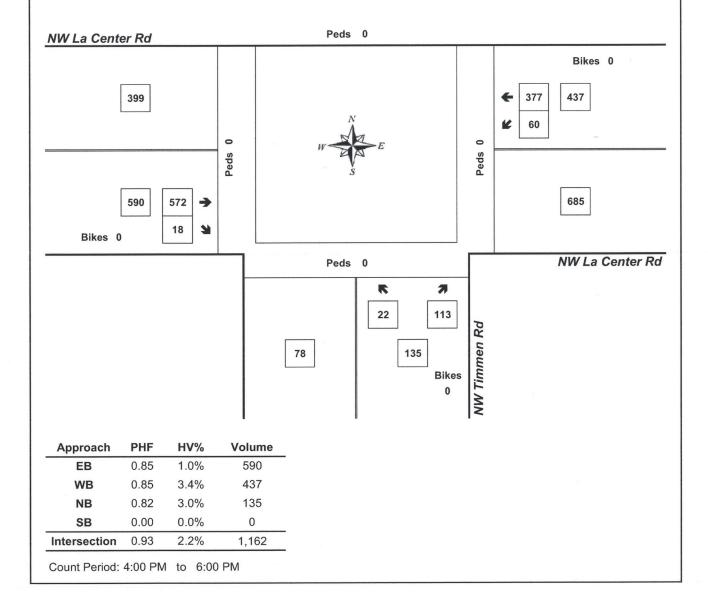


Clay Carney (503) 833-2740

NW Timmen Rd & NW La Center Rd

4:30 PM to 5:30 PM Wednesday, May 08, 2019

Bikes 0



APPENDIX B COLLISION DATA

CITY STREET INTERSECTIONS

4th St @ Highland Rd - No Reported Crashes

4th St @ Pacific Highway

LaCenter Rd @ Timmen Rd - No Reported Crashes

Lockwood Creek Rd @ Spruce Ave

Lockwood Creek Rd @ John Storm Rd - No Reported Crashes

COUNTY ROAD INTERSECTIONS

Lockwood Creek Rd (Co Rd # 94450, mp 7.360 - 7.380 - East Leg & City St - West Leg) @ 24th Ave (Co Rd # 60430, mp 0.470 - 0.490) - No Reported Crashes

OFFICER REPORTED CRASHES THAT OCCURRED at OR in the vicinity of MULTIPLE INTERSECTIONS IN THE CITY OF LA CENTER

11/04/2018 - 11/04/2021 See 2nd tab below for road info

or planning the safety enhancement of potential crash sites, hazardous roadway conditions, or railway-highway crossings are not subject to discovery or admitted Under 23 U.S. Code § 148 and 23 U.S. Code § 409, safety data, reports, surveys, schedules, lists compiled or collected for the purpose of identifying, evaluating, into evidence in a Federal or State court proceeding or considered for other purposes in any action for damages arising from any occurrence at a location mentioned or addressed in such reports, surveys, schedules, lists, or data.

			EFERENCE	POINT	NAME		
	COMP	DIR	FROM	REF	POINT FT POINT		
-			Ξ	or	ㅂ		
_		DIST	FROM	REF or	POINT		
		CO ONLY	INTERSECTING FROM MI FROM REFERENCE	COUNTY ROAD	MILEPOST		
				INTERSECTING	TRAFFICWAY	E SPRUCE AVE	NW PACIFIC HWY
-			A	\	В		
					NUMBER MILEPOST B		
				BLOCK	NUMBER	1800	0
					PRIMARY TRAFFICWAY	La Center NE LOCKWOOD CREEK RD	La Center W 4TH ST
(J					CITY	La Center	La Center
					COUNTY	Clark	Clark
to the state of th					JURISDICTION COUNTY	City Street	City Street

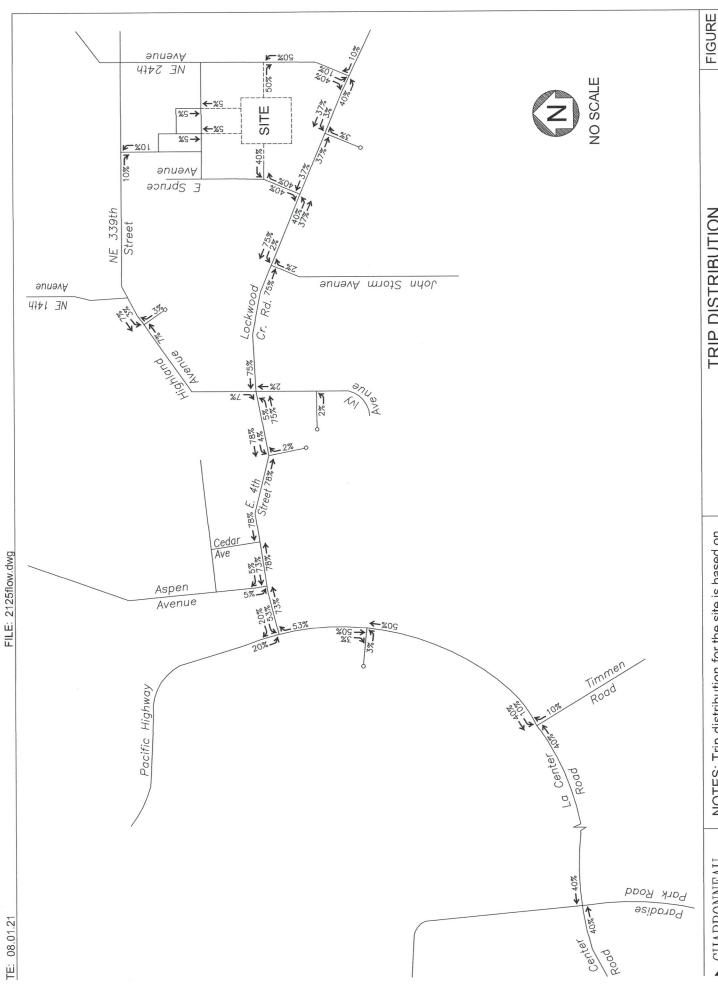
					JUNCTION RELATIONSHIP	At Intersection and Not Related	18:21 No Apparent Injury 0 0 2 0 0 Passenger Car Not Stated Circulating Roundabout
				VEHICLE 2	TYPE		Not Stated
				VEHICLE 1	TYPE	Passenger Car	Passenger Car
#	8	_	FVEK	NAEDE	JTHSS	0	0
	#	# # #	ш	0	S	0	0
		#	>	Ш	I	1	2
		#	<u>ц</u>	7	_	0	
		##	_	_	_	-	\vdash
				MOST SEVERE	INJURY TYPE	12:58 No Apparent Injury 0 0 1 0 0 Passenger Car	No Apparent Injury
					TIME	12:58	18:21
					DATE	EB67457 08/02/2021	E996098 12/14/2019
				REPORT	NUMBER	EB67457	E996098
		SR ONLY	HISTORY /	SUSPENSE REPORT	QNI	No	No

		VEHICLE 1 ACTION	Overtaking and Passing	Making Right Turn
	FIRST COLLISION TYPE / OBJECT	STRUCK	Fence	Dark-Street Lights On From opposite direction - all others Making Right Turn
		CONDITION LIGHTING CONDITION	Daylight	Dark-Street Lights On
ROADWAY	SURFACE	CONDITION	Dry	Wet
		WEATHER	Clear or Partly Cloudy	Fog or Smog or Smoke

	VEHICLE 1	VEHICLE 1	IICLE 1 VEHICLE 1 VEHICLE 2 VEHICLE 2	VEHICLE 2	MV DRIVER	
	COMPASS	COMPASS	COMPASS COMPASS COMPASS	COMPASS	CONTRIBUTING	
	DIRECTION	DIRECTION	DIRECTION	DIRECTION	DIRECTION DIRECTION DIRECTION DIRECTION CIRCUMSTANCE 1	MV DRIVER CONTRIBUTING
VEHICLE 2 ACTION	FROM	T0	FROM	TO	(UNIT 1)	CIRCUMSTANCE 1 (UNIT 2)
	West	East			Improper Passing	
Going Wrong Way on Divided Hwy South	South	Northeast			None	Other Contributing Circ Not Listed

	WA STATE WA STATE	WA STATE
	PLANE	PLANE
	SOUTH - X SOUTH - Y	SOUTH - Y
FIRST IMPACT LOCATION (City, County & Misc	2010 -	2010 -
Trafficways - 2010 forward)	FORWARD FORWARD	FORWARD
Past the Outside Shoulder of Primary Trafficway	1092222.94 199937.35	199937.35
Lane of Primary Trafficway	1087021.99 200383.21	200383.21

APPENDIX C IN-PROCESS TRAFFIC



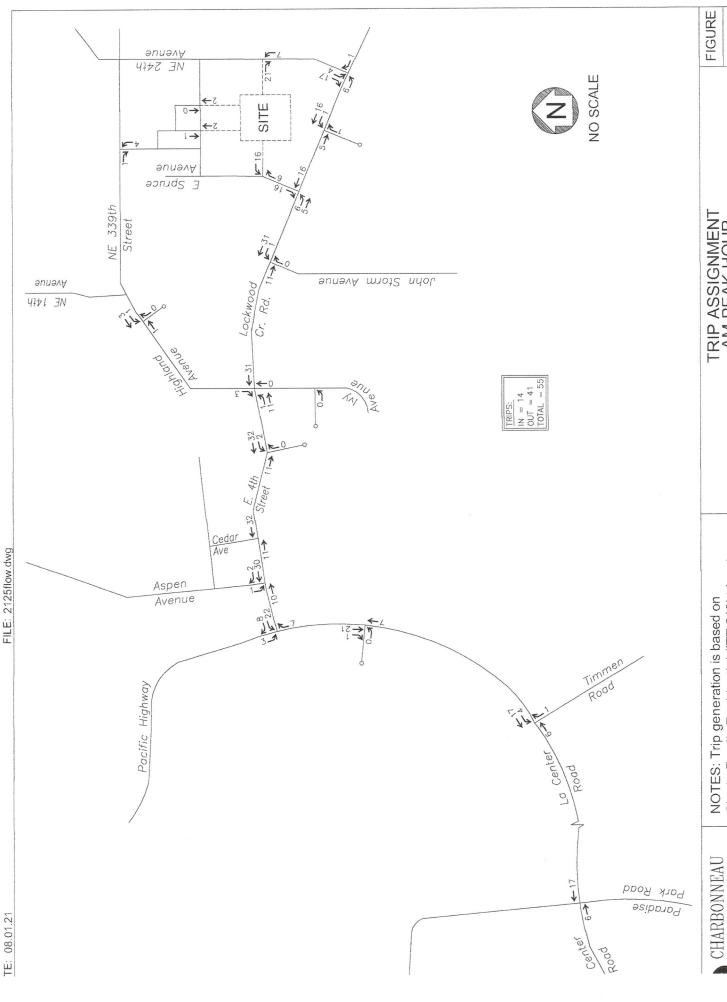
TRIP DISTRIBUTION AM PEAK HOUR & PM PEAK HOUR LOCKWOOD MEADOWS

NOTES: Trip distribution for the site is based on engineering judgment.

CHARBONNEAU ENGINEERING LLC

CT: 21-25

4

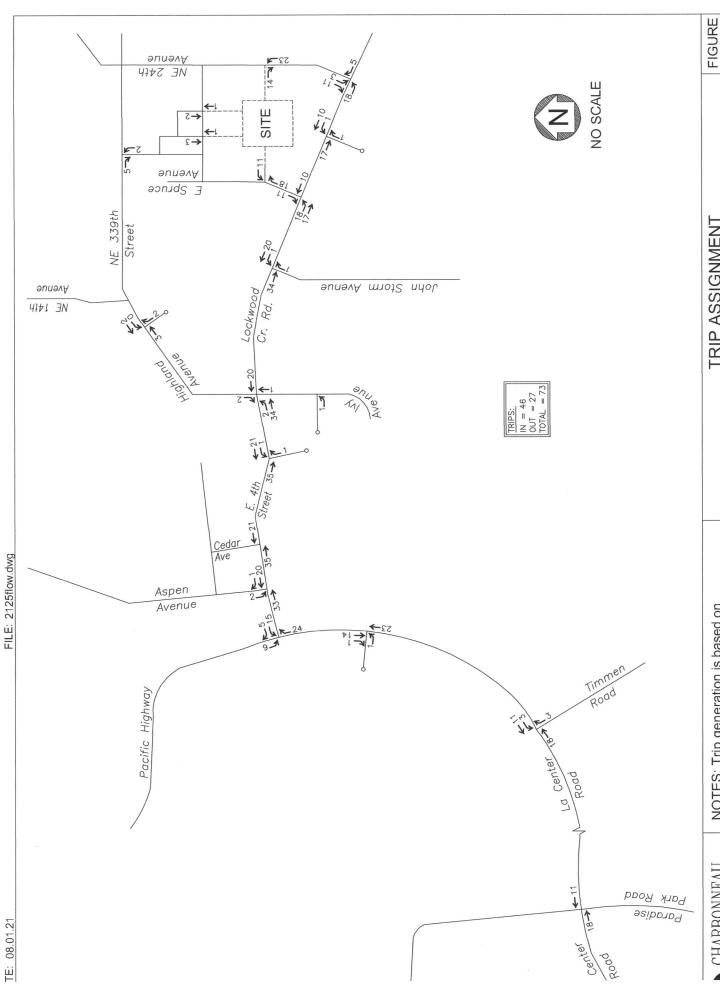


LOCKWOOD MEADOWS TRIP ASSIGNMENT AM PEAK HOUR

5a

NOTES: Trip generation is based on Single-Family Residential (ITE 210) trip rates.

ENGINEERING LLC 7.T. 91-95



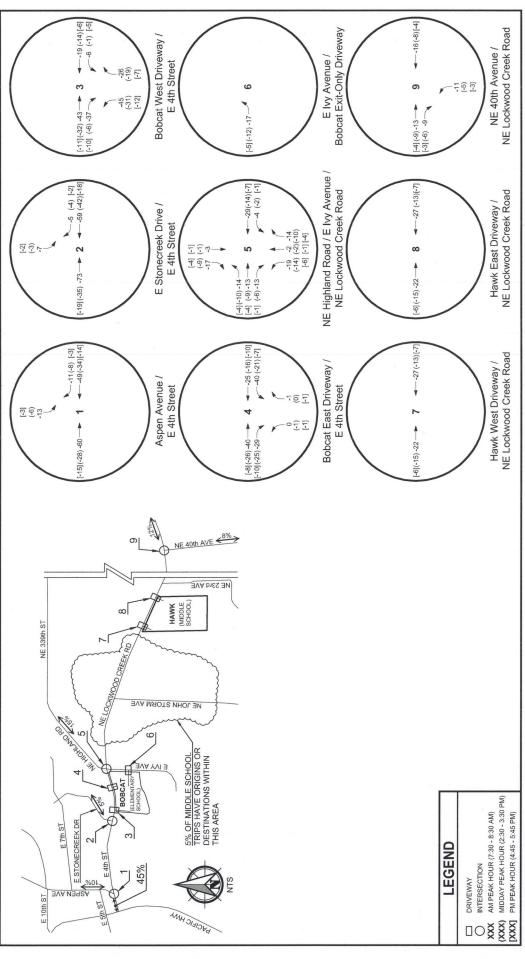
LOCKWOOD MEADOWS TRIP ASSIGNMENT PM PEAK HOUR

2b

NOTES: Trip generation is based on Single-Family Residential (ITE 210) trip rates.

CHARBONNEAU ENGINEERING LLC

CT: 21-25



Existing Middle School Trip Distribution and Adjustments

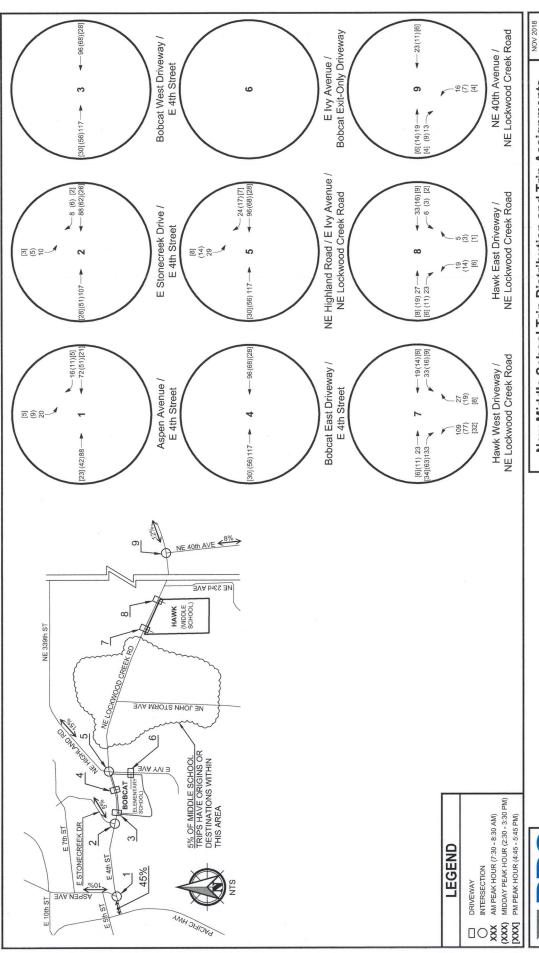
La Center Middle School

9

Traffic Impact Analysis November 2018

N PBS

PBS Engineering and Environmental Inc. Project No. 71282.000



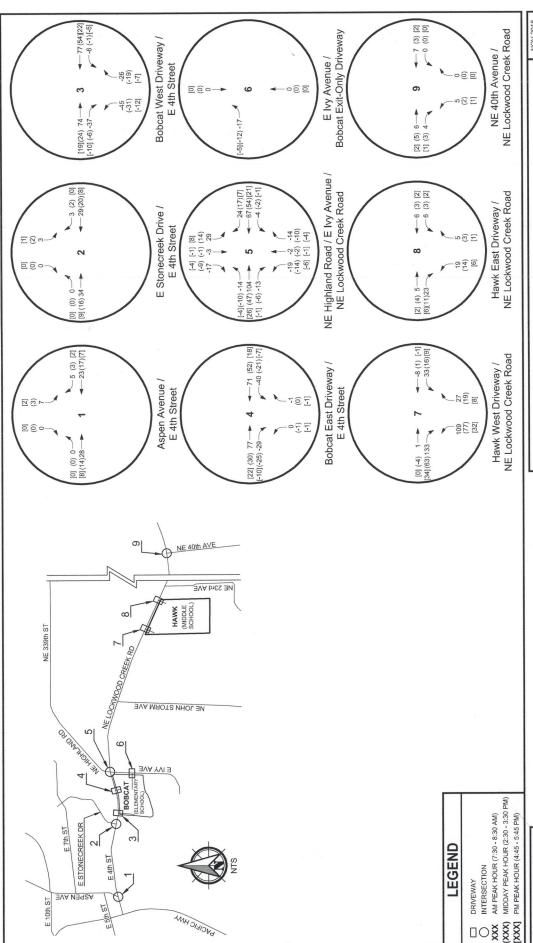


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PBS Engineering and Environmental Inc. Project No. 71282.000

Traffic Impact Analysis November 2018





Traffic Impact Analysis November 2018



[XXX]

PBS Engineering and Environmental Inc. Project No. 71282.000



MEMORANDUM

Date:

March 2, 2020

To:

Mike Odren, RLA Associate Principal Olson Engineering, Inc. 222 East Evergreen Blvd Vancouver WA 98660

From:

Frank Charbonneau, PE, PTOE

Subject:

Trip Generation Assessment

Minit Management Development

NW Paradise Park Road, La Center

FL2024

This memo will serve as the trip generation assessment documenting the number of vehicular trips that will be produced by the proposed Minit Management development. The four acre site at address #2814 NW 319th Street is located in the northeast quadrant of NW La Center Road and the I-5 northbound on-ramp.

The development project will demolish the existing convenience store and gas station facilities and construct several new buildings consisting of 11,600 square feet of general retail, fast foot restaurant with drive-through totaling 2,800 square feet, convenience market with coffee drive-through totaling 4,510 square feet, and a 101 unit hotel. Parking on the site for 184 spaces will be provided, including eight ADA parking stalls. A copy of the project's site plan is attached to this memo.

The site we be served by three driveway accesses connecting to the perimeter road (NW Paradise Park Road) on the property's north and east sides. The nearest major intersections include NW La Center Road at the I-5 northbound off-ramp which is configured as a round-about and NW Paradise Park Road at NW La Center Road. This intersection is controlled by stop signing on the northbound Paradise Park Road approach and on the southbound Paradise Road approach.

The City of La Center issued a pre-application conference report (2019-018-PAC) dated June 11, 2019 documenting the application's process and requirements. The staff report detailed that the development agreement between the City and Minit Management LLC dated March 2016 vested a total of 199 PM peak hour trips for the site. As a result it was necessary to submit a trip generation assessment to verify the trip projection.

The number of trips were calculated based on the proposed building uses and sizes. Trip credits were applied for the existing facilities that will be demolished including the convenience market and gas station and a cardlock fueling station. The trip calculations were determined for the weekday average daily traffic (ADT) and the weekday AM and PM peak hours.

Phone: (503) 293-1118

The analysis used the <u>ITE Trip Generation</u> manual (10th edition, year 2017).

For the proposed site uses several ITE land use categories were applied including #310 (Hotel), #820 (shopping center), #852 (convenience market), #934 (fast food restaurant with drive-through), and #938 (coffee drive-through). For the existing uses ITE code #853 for convenience market was used and historical rates for Pacific Pride Cardlock were applied for the cardlock fueling station.

A summary of the site's trip generation is provided in the following tables. Table 1 provides the trip generation for the site's existing uses. Table 2 provides the trip generation for the proposed site uses. Table 3 lists the net site trips for the development.

Table 1 Existing Land Uses Trip Generation Summary

			Weekday						
ITE Land Use	Units	ADT	AM	Peak H	our	PM	Peak H	lour	
		ADI	Total	Enter	Exit	Total	Enter	Exit	
Convenience Mkt with Gas (#853)	6 fueling								
Generation Rate ¹	posiitons	322.50	20.76	50%	50%	23.04	50%	50%	
Total Driveway Trips	positions	1,935	125	63	62	138	69	69	
Pass-By Trips ² (AM Peak=63%; PM Peak=66%)			79	40	39	91	46	45	
New Site Trips			46	23	23	47	23	24	
Cardlock Fueling Station	12 fueling								
Generation Rate ³	positions		4.44	50%	50%	2.96	50%	50%	
Total Driveway Trips	positions	1445	53	27	26	36	18	18	
Pass-By Trips ² (AM Peak=58%; PM Peak=42%)			31	16	15	15	8	7	
New Trips			22	11	11	21	10	11	
Total Site Trips			178	90	88	174	87	87	
Pass-by Trips			110	56	54	106	54	52	
New Trips ⁴		3,380	68	34	34	68	33	35	

¹ Source: *Trip Generation*, 10th Edition, ITE, 2017, average rates.

² Pass-by percentage based on *Trip Generation Handbook*, *3nd Edition*, ITE, 2017.

³ Source: Independent surveys at Tarr Inc. Pacific Pride. AM trip rate = 1.5x calculated PM trip rate, ADT = 70% of ITE #944 Gas Station Rate

⁴ New Trips = Total Trips - Internal Trips - Pass-by Trips.

Table 2 Proposed Land Uses Trip Generation Summary

Table 2 Proposed Land Oses Trip Generation		,		We	ekday			
ITE Land Use	Units	[AM F	eak Ho		PM	Peak H	our
THE Edita GGG		ADT	Total	Enter	Exit	Total		Exit
Convenience Mkt [Open 15-16 hours] (#852)	4,410 sq.	T U	21.0					
Generation Rate 1,2	ft.	345.70	31.02	50%	50%	34.57	49%	51%
Total Driveway Trips		1,525	137	69	68	152	74	78
Internal Trips ³ (AM Peak=16%; PM Peak=36%)			22	11	11	55	27	28
Pass-By Trips ⁴ (AM Peak=63%; PM Peak=66%)			72	36	36	64	31	33
New Site Trips		1,525	43	22	21	33	16	17
Shopping Center (#820)	11,600							
Generation Rate ²	sq. ft.	37.75	0.94	62%	38%	3.81	48%	52%
Total Driveway Trips		438	11	7	4	44	21	23
Internal Trips ³ (AM Peak=16%; PM Peak=36%)			2	1	1	16	8	8
Pass-By Trips 4 (AM Peak=N/A; PM Peak=34%)						10	5	5
New Site Trips ⁴		438	9	6	3	18	8	10
Hotel (#310)	101							
Generation Rate ²	rooms	8.36	0.47	59%	41%	0.60	51%	49%
Total Driveway Trips		844	47	28	19	61	31	30
Internal Trips ³ (AM Peak=16%; PM Peak=36%)			8	4	4	22	11	11
New Site Trips			39	24	15	39	20	19
Fast-Food with Drive-Through (#934)	2,800 sq.							
Generation Rate ²	ft.	470.95	40.19	51%	49%	32.67	52%	48%
Total Driveway Trips		1,319	113	58	55	91	48	43
Internal Trips ³ (AM Peak=16%; PM Peak=36%)			19	10	9	33	17	16
Pass-By Trips ⁴ (AM Peak=49%; PM Peak=50%)			46	24	22	29	15	14
New Trips			48	24	24	29	16	13
Coffee/Donut Shop with Drive-Through	100							
& No Indoor Seating (#938)	sq. ft.							
Generation Rate ²		2000.00	337.04	50%	50%	83.33		50%
Total Driveway Trips		200	34	17	17	8	4	4
Internal Trips ³ (AM Peak=16%; PM Peak=36%)		0	6	3	3	3	2	1
Pass-By Trips 4,5 (AM Peak=83%; PM Peak=83%)		166	23	12	11	4	2	2
New Site Trips		34	5	2	3	1	0	1
Total Site Trips		4,326	342	179	163	356	178	178
Internal Trips			57	29	28	129	65	64
Pass-by Trips			141	72	69	107 120	53 60	54 60
New Trips	rin rato		144	78	66	120	00	00

¹ ADT trip rate estimated as ten times the PM peak hour trip rate.

² Source: *Trip Generation*, 10th Edition, ITE, 2017, average rates.

³ Internal capture calculated with unconstrained internal capture rates presented in the Center for Urban Transportation Research (CUTR) Trip Internalization in Multi-Use Developments, April 2014, FDOT.

⁴ Pass-by percentage based on Trip Generation Handbook, 3nd Edition, ITE, 2017.

 $^{^{5}\,}$ The weekday PM peak pass-by rate used to calculate the daily and weekday AM peak pass-by trips.

⁶ New Trips = Total Trips - Internal Trips - Pass-by Trips.

Table 3 presents the net trip generation results (proposed site trips – existing site trips) for the development project. When the new facility is developed it is projected that the site will generate a net of 76 trips in the AM peak hour 52 trips in the PM peak hour. The ADT is projected to increase by 946 trips per day.

Table 3 Net New Trips

			Weekday	Peak Hour			Weekday
Site Uses	А	M Peak Hou	ur	Р	M Peak Hοι	ır	ADT
	Total						
Proposed Site 1	144	78	66	120	60	60	4,326
Existing Site ²	-68	-34	-34	-68	-33	-35	3,380
Net New Trips 3	76	44	32	52	27	25	946

¹ Refer to Table 2.

It is recommended that the City of La Center support the proposed development without the application of traffic impact fees as the projected number of site trips falls below the vested number of peak hour trips (199 trips) identified in the City's development agreement with Minit Management.

If you should need any additional traffic engineering support on this project or if there are any further questions, please contact Frank Charbonneau, PE, PTOE at 503.293.1118 or email Frank@CharbonneauEngineer.com.

Attachment

Site Plan

² Refer to Table 1.

³ Net New Trips = Proposed Site Trips - Existing Site Trips.

APPENDIX D BREEZE CREEK PROJECT





CITY OF LA CENTER | BREZEE CREEK CULVERT REPLACEMENT & 4TH STREET WIDENING FAQ

Thank you for your comments and questions about the Brezee Creek Culvert Replacement and 4th Street Widening Project. The topics listed below reflect feedback received to date during public meetings as well as those regarding the Virtual Open House released the week of October 19, 2020. This document will be updated periodically to address new frequently asked questions.







CITY OF LA CENTER | BREZEE CREEK CULVERT REPLACEMENT & 4TH STREET WIDENING FAQ

TRAFFIC SIGNAL VS. ROUNDABOUT



OR



Comment: Include the roundabout alternative in the scope of the 4th Street widening project. If the budget is unavailable for the roundabout alternative at the time of the 4th Street widening, the do nothing alternative should be chosen until funds are available. Signalization poses a significantly higher statistical risk to pedestrians than a stop sign controlled intersection. An increased risk to pedestrians is unacceptable for a primary intersection connecting the parks and schools of La Center.

Background

- Safety is paramount to both the design team and City staff who are working on this project.
- The original realignment of Highland Road and 4th Street, which took place in 2008, was designed to support a future traffic signal at this intersection.
- The posted speed limit of 4th Street is 25 mph with a reduction to 20 mph within the school zone while children are present. For a low-speed corridor of this type, both a signal and roundabout effectively provide a safe configuration for all users.

Design Process

- As part of the intersection design process completed during the current Brezee Creek Culvert Replacement/4th Street Widening Project, a traffic study and report were completed.
- Level of Service (LOS) is a measure of vehicular traffic flow, reflecting indicators such as speed, travel time, freedom to maneuver, traffic interruptions, comfort, and convenience.
- The LOS for the Highland Road and 4th Street intersection is projected to be LOS F (failing) by 2040 if left as-is.
- According to LOS guidelines set forth by the City of La Center, if the LOS exceeds D, the intersection fails, and an intersection improvement is required.

Comparison

- A decision to use a signal or roundabout to address a
 deficient intersection, in this case Highland Road and
 4th Street, requires numerous factors to be considered.
 For the Highland Road and 4th Street intersection,
 both alternatives (roundabout and signalization) would
 provide the necessary safety and LOS improvements
 required by City guidelines.
- However, for this specific intersection in a low-speed corridor, a signal provides safe crossing routes for pedestrians and bicyclists, particularly school children by requiring traffic to completely stop, offering visibility, and providing an allotted amount of time for pedestrian and bicyclist passage through the intersection.
- A signal provides safety and LOS improvements with a significantly lower construction cost, reduced property impacts and less right-of-way acquisition (the City purchasing property from private owners or public agencies).
- The analysis of available accident data completed as part of the Project development at this intersection reveals that only a single incident has occurred. The installation of a signal could effectively prevent this type of accident in the future.

APPENDIX E LEVEL OF SERVICE COMPUTER PRINTOUTS

	I V	WO-WAY STOP								
General Information				formatio	on					
Analyst Agency/Co. Date Performed Analysis Time Period	DSK Kelly Engi 2/20/2022 AM Peak			Jurisdiction Analysis Year			Lockwood Cr. Rd. & 24th Av City of La Center 2022			
	sting									
East/West Street: Lockw		i .	North/South Street: 24th Ave. Study Period (hrs): 0.25							
Intersection Orientation:	East-West		Study F	eriod (hrs	s): 0.25					
Vehicle Volumes and	Adjustments									
Major Street		Eastbound				Westbou	nd			
Movement	1 1	2	3		4	5 T		6 R		
	L	T	R		L 1	119		8		
Volume (veh/h) Peak-Hour Factor, PHF	0.82	67 0.82	0.82		0.82	0.82		<u> </u>		
Hourly Flow Rate, HFR	2	81	0.82		1	145		9		
(veh/h) Percent Heavy Vehicles	50				0					
Median Type			Undivided							
RT Channelized			0					0		
Lanes	0	1	0		0	1		0		
Configuration	LTR	<u> </u>			LTR					
Upstream Signal		0				0				
Minor Street	Northbound					Southbou	ınd			
Movement	7	8	9		10	11		12		
Wovernone	L	T	R		L	T		R		
Volume (veh/h)	0	0	0		3	0		6		
Peak-Hour Factor, PHF	0.82	0.82	0.82		0.82	0.82		0.82		
Hourly Flow Rate, HFR (veh/h)	0	0	0		3	0		7		
Percent Heavy Vehicles	0	0	0		0	0		17		
Percent Grade (%)		0				0				
Flared Approach		N				N				
Storage		0				0				
RT Channelized			0					0		
Lanes	0	1	0		0	1		0		
Configuration		LTR				LTR				
Delay, Queue Length, a	nd Level of Ser	vice								
Approach	Eastbound	Westbound		Northbou	nd	5	Southbound	1		
Movement	1	4	7	8	9	10	11	12		
Lane Configuration	LTR	LTR		LTR			LTR			
v (veh/h)	2	1		0			10			
C (m) (veh/h)	1180	1529					812			
v/c	0.00	0.00					0.01			
95% queue length	0.01	0.00					0.04			
Control Delay (s/veh)	8.1	7.4					9.5			
LOS	А	A					A			

Approach Delay (s/veh)

Approach LOS

9.5

Α

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O			Cita In	formatio	n			
Analyst Agency/Co. Date Performed Analysis Time Period	DSK Kelly Engir 2/20/2022 AM Peak F		Interse Jurisdic Analys	ction ction	n	Lockwood City of La 2025	Cr. Rd. & : Center	24th Ave
Project Description Yea	r 2025 w/o Proje	ct						
East/West Street: Lockw	ood Creek Road				et: 24th Av	e.		
Intersection Orientation:	East-West		Study F	eriod (hrs): 0.25			
Vehicle Volumes and	Adjustments							
Major Street		Eastbound					nd	
Movement	1	2	3		4	5		6
	L	T	R		L	T 100		R
Volume (veh/h)	18	81	0		1	138		9 0.82
Peak-Hour Factor, PHF Hourly Flow Rate, HFR	0.82	0.82	0.82		0.82	0.82		
(veh/h)	21	98	0		1	168		10
Percent Heavy Vehicles	50			11	0			
Median Type			Undivided					
RT Channelized			0					0
Lanes	0	1	0		0	1		0
Configuration	LTR				LTR			
Upstream Signal		0				0		
Minor Street		Northbound				Southbou	nd	
Movement	7	8	9		10	11		12
	L	T	R		L	T		R
Volume (veh/h)	0	0	0		7	0		24
Peak-Hour Factor, PHF	0.82	0.82	0.82 0.82		0.82	0.82		0.82
Hourly Flow Rate, HFR (veh/h)	0	0	0		8	0		
Percent Heavy Vehicles	0	0	0		0	0		17
Percent Grade (%)		0				0		
Flared Approach		N				N		
Storage		0				0		
RT Channelized			0					0
Lanes	0	1	0		0	1		0
Configuration		LTR				LTR		
Delay, Queue Length, ar	nd Level of Serv	rice						
Approach	Eastbound	Westbound		Northbour	nd	S	outhbound	
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LTR	LTR		LTR			LTR	
v (veh/h)	21	1		0			37	
C (m) (veh/h)	1155	1508					779	
v/c	0.02	0.00					0.05	
95% queue length	0.06	0.00				10.00	0.15	
Control Delay (s/veh)	8.2	7.4					9.9	
LOS	A A	A					A	
Approach Delay (s/veh)							9.9	-
						+	A	
Approach LOS	orida, All Rights Rese			HCS+™ Ve			erated: 2/20/2	

	1	WO-WAY STOP	CONTING	L SON	IIVI	VIV I			
General Information			Site In	format	tion				
Analyst Agency/Co. Date Performed Analysis Time Period	DSK Kelly Engi 2/20/2022 AM Peak		Intersection Jurisdiction Analysis Year Lockwood Cr. Rd. & 24th A City of La Center 2025						24th Ave
Project Description Yea	ar 2025 with Proj	ect							
East/West Street: Lockw	ood Creek Road	1				24th Ave	9.		
Intersection Orientation:	East-West		Study F	Period (h	nrs):	0.25			
Vehicle Volumes and	Adjustments								
Major Street		Eastbound					Westbou	nd	
Movement	1	2	3			4	5		6
	L	T	R			L	T		R
Volume (veh/h)	18	81	12			1	138		9
Peak-Hour Factor, PHF	0.82	0.82	0.82			0.82	0.82		0.82
Hourly Flow Rate, HFR (veh/h)	21	98	14			1	168		10
Percent Heavy Vehicles	50					0			
Median Type				Undivi	ided				
RT Channelized			0						0
Lanes	0	1	0		0		1		0
Configuration	LTR					LTR			
Upstream Signal		0					0		
Minor Street		Northbound					Southbou	ınd	
Movement	7	8	9			10	11		12
	L	Т	R			L	T		R
Volume (veh/h)	33	0	4			0		24	
Peak-Hour Factor, PHF	0.82	0.82	0.82	0.82 0.82		0.82	0.82		0.82
Hourly Flow Rate, HFR (veh/h)	40	0	4			8	0		29
Percent Heavy Vehicles	0	0	0			0	0		17
Percent Grade (%)		0					0		
Flared Approach		N					N		
Storage		0					0		
RT Channelized			0						0
Lanes	0	1	0			0	1		0
Configuration		LTR					LTR		
Delay, Queue Length, a	nd Level of Sen	/ice							
Approach	Eastbound	Westbound		Northbo	und		5	Southbound	1
Movement	1	4	7	8		9	10	11	12
Lane Configuration	LTR	LTR		LTR				LTR	
v (veh/h)	21	1		44				37	
C (m) (veh/h)	1155	1490		611				776	
v/c	0.02	0.00		0.07	. 1			0.05	
95% queue length	0.06	0.00		0.23				0.15	
Control Delay (s/veh)	8.2	7.4		11.3				9.9	
LOS	Α	Α		В				Α	
Approach Delay (s/veh)				11.3				9.9	
Approach LOS			В		A				

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	• •	WO-WAY STOP								
Analyst Agency/Co. Date Performed Analysis Time Period	DSK Kelly Engi 2/20/2022 PM Peak		Jurisdio	Intersection Jurisdiction Analysis Year			Lockwood Cr. Rd. & 24th Ave City of La Center 2022			
Project Description Exis										
East/West Street: Lockw		1			t: 24th Av	e				
Intersection Orientation:	East-West		Study Period (hrs): 0.25							
Vehicle Volumes and	Adjustments					147 (1				
Major Street	1	Eastbound	3		4	Westbour 5	nd	6		
Movement	1 L		R			T 7		R		
Volume (veh/h)	11	197	0		1	108		16		
Peak-Hour Factor, PHF	0.86	0.86	0.86		0.86	0.86		0.86		
Hourly Flow Rate, HFR (veh/h)	12	229	0		1	125		18		
Percent Heavy Vehicles	0				0					
Median Type				Undivide	d					
RT Channelized			0					0		
Lanes	0	1	0		0	1		0		
Configuration	LTR				LTR					
Upstream Signal		0				0				
Minor Street		Northbound			Southbou	ınd				
Movement	7	8	9		10	11		12		
	L	Т	R		L	T		R		
Volume (veh/h)	0	0	0		6	0		4		
Peak-Hour Factor, PHF	0.86	0.86	0.86 0.86		0.86	0.86		0.86		
Hourly Flow Rate, HFR (veh/h)	0	0	0		6	0		4 25		
Percent Heavy Vehicles	0	0	0		0	0		25		
Percent Grade (%)		0	Т			0				
Flared Approach		N				N				
Storage		0				0	_	0		
RT Channelized			0			1		0		
Lanes	0	1	0		0					
Configuration		LTR				LTR				
Delay, Queue Length, ar						T .				
Approach	Eastbound	Westbound		Northboun			Southbound			
Movement	1	4	7	8	9	10	11	12		
Lane Configuration	LTR	LTR		LTR			LTR			
v (veh/h)	12	1		0			10			
C (m) (veh/h)	1452	1351					657			
v/c	0.01	0.00					0.02			
95% queue length	0.02	0.00					0.05			
Control Delay (s/veh)	7.5	7.7					10.6			
LOS	A	A					В			
Approach Delay (s/veh)					1		10.6			
						+	B			
Approach LOS	orida All Pights Res			HCS+TM Ver			erated: 2/20/2	1000 0 11		

General Information			Site In	ormatio	n				
Analyst Agency/Co. Date Performed Analysis Time Period	DSK Kelly Engii 2/20/2022 PM Peak I		Intersed Jurisdic Analysi	ction ction		Lockwood City of La (2025		24th Ave	
	r 2025 w/o Proje	ect							
East/West Street: Lockw			North/S	outh Stree	et: 24th Av	е.			
Intersection Orientation:			Study P	eriod (hrs)): 0.25				
Vehicle Volumes and	Adjustments								
Major Street		Eastbound				Westbour	nd		
Movement	1	2	3		4	5		6	
	L	T	R		L	T		R	
Volume (veh/h)	30	212	0		1	117		23	
Peak-Hour Factor, PHF	0.86	0.86	0.86		0.86	0.86	- (0.86	
Hourly Flow Rate, HFR (veh/h)	34	246	0		1	136		26	
Percent Heavy Vehicles	0				0				
Median Type				Undivide	ed				
RT Channelized			0					0	
Lanes	0	1	0		0	1		0	
Configuration	LTR				LTR				
Upstream Signal		0				0			
Minor Street	1	Northbound		i		Southbou	nd		
Movement	7	8	9		10	11		12	
Wordmont	L	T	R		L	Т		R	
Volume (veh/h)	0	0	0		9	0		16	
Peak-Hour Factor, PHF	0.86	0.86	0.86	0.86 0.86		0.86	0.86		
Hourly Flow Rate, HFR (veh/h)	0	0	0		10	0		18	
Percent Heavy Vehicles	0	0	0		0	0		25	
Percent Grade (%)		0				0			
Flared Approach		N				N			
Storage		0				0			
RT Channelized			0					0	
Lanes	0	1	0		0	1		0	
Configuration		LTR				LTR			
	nd Lovel of Com								
Delay, Queue Length, a	Eastbound	Westbound		Northbour	nd	T s	outhbound		
Approach Movement	1	4	7	8	9	10	11	12	
Lane Configuration	LTR	LTR		LTR			LTR		
		1		0	1		28		
v (veh/h)	34 1429	1332			-		676		
C (m) (veh/h)					-			+	
v/c	0.02	0.00					0.04	-	
95% queue length	0.07	0.00					0.13		
Control Delay (s/veh)	7.6	7.7					10.6		
LOS	Α	Α					В		
Approach Delay (s/veh)							10.6		
							B		
Approach LOS		erved					erated: 2/20/2		

	11	WO-WAY STOP	CONTRO	L SOIVI	INIMI				
General Information			Site In	formati	ion				
Analyst Agency/Co. Date Performed Analysis Time Period	DSK Kelly Engli 2/20/2022 PM Peak I		Jurisdie	Intersection Jurisdiction Analysis Year			Lockwood Cr. Rd. & 24th Ave City of La Center 2025		
Project Description Yea	ar 2025 with Proje	ect							
East/West Street: Lockw		1		outh Str				· · · · · · · · · · · · · · · · · · ·	
Intersection Orientation:	East-West		Study F	eriod (h	rs): <i>0.</i>	25			
Vehicle Volumes and	Adjustments								
Major Street		Eastbound	1 0				Westbou	nd T	6
Movement	1 1	2 	3 R		4		5 T		6 R
Volume (veh/h)	30	212	38		4		117		23
Peak-Hour Factor, PHF	0.86	0.86	0.86		0.8		0.86		0.86
Hourly Flow Rate, HFR (veh/h)	34	246	44		4		136		26
Percent Heavy Vehicles	0				0				
Median Type		Undivided							
RT Channelized			0					0	
Lanes	0	1	0		0	2 2	1		0
Configuration	LTR				LTI	7			
Upstream Signal		0					0		
Minor Street		Northbound					Southbou	ınd	
Movement	7	8	9		10	0	11		12
	L	T	R		L		Т		R
Volume (veh/h)	23	0		2			0		16
Peak-Hour Factor, PHF	0.86	0.86	0.86	-	0.8	6	0.86		0.86
Hourly Flow Rate, HFR (veh/h)	26	0	2		0		0		18 25
Percent Heavy Vehicles	0		0				0		25
Percent Grade (%)		0	1				l N		
Flared Approach		N					0		
Storage	-	0	0				0		0
RT Channelized	0	1	0		0		1		0
Lanes Configuration		LTR	-				LTR		
	ad Laval of Com								
Delay, Queue Length, a	Eastbound	Westbound		Northbo	und		Τ	Southbound	1
Approach			7	8	unu	9	10	11	12
Movement	1	4	/			9	10		12
Lane Configuration	LTR	LTR		LTR	_		-	LTR	-
v (veh/h)	34	4		28				28	
C (m) (veh/h)	1429	1283		475				660	
v/c	0.02	0.00		0.06				0.04	
95% queue length	0.07	0.01		0.19	_			0.13	
Control Delay (s/veh)	7.6	7.8	13.1					10.7	
LOS	Α	Α		В				В	
Approach Delay (s/veh)				13.1				10.7	
Approach LOS			В						

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		WO-WAY STOP						
General Information			Site In	format	tion			
Analyst	DSK		Interse				Cr. Rd. &	Sprice
Agency/Co.	Kelly Engi		Jurisdio			City of La	Center	
Date Performed	2/21/2022		Analysi	is Year		2022		
Analysis Time Period	AM Peak	Hour						
	sting		1					
East/West Street: Lockw					reet: Spruce	Ave.		
ntersection Orientation:			Study P	eriod (f	nrs): 0.25			
/ehicle Volumes and	Adjustments							
Major Street		Eastbound	1 0		4	Westbou	nd 	6
Movement	1	2 T	3 R		4 L	5 T		6 R
/aluma (vah/h)	19	81	K	-	L	131		4
/olume (veh/h) Peak-Hour Factor, PHF	0.83	0.83	1.00		1.00	0.83		0.83
Hourly Flow Rate, HFR								
veh/h)	22	97	0		0	157		4
Percent Heavy Vehicles	0				0			
Median Type			_	Undiv	ided			
RT Channelized			0					0
anes	0	1	0		0	1		0
Configuration	LT							TR
Upstream Signal		0				0		
Minor Street		Northbound				Southbou	ınd	
Movement	7	8	9		10	11		12
	L	T	R		L	T		R
/olume (veh/h)				1 00 000				16
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00 0.83		1.00		0.83
Hourly Flow Rate, HFR veh/h)	0	0	0		1	0		19
Percent Heavy Vehicles	0	0	0		0	0		0
Percent Grade (%)		0				0		
Flared Approach		N				N		
Storage		0				0		
RT Channelized			0					0
Lanes	0	0	0		0	0		0
Configuration						LR		
Delay, Queue Length, a	nd Level of Sen	vice						
Approach	Eastbound	Westbound	1	Northbo	ound	5	Southbound	
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT						LR	
v (veh/h)	22						20	
C (m) (veh/h)	1430						879	
//c	0.02						0.02	
95% queue length	0.02					1	0.07	1
							9.2	1
Control Delay (s/veh)	7.6							+
				1	1	1	A	1
LOS	Α							
					I		9.2 A	

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	•	WO-WAY STOP	,					
General Information Analyst Agency/Co. Date Performed	DSK Kelly Engi 2/21/2022		Site Inf Intersect Jurisdict Analysis	ction	ion	Lockwood City of La 2025		Sprice
Analysis Time Period	AM Peak	Hour						
Project Description Yea	ar 2025 w/o Proje	ect						
East/West Street: Lockw	ood Creek Rd.				eet: Spruce	Ave.		
ntersection Orientation:	East-West		Study P	eriod (h	rs): 0.25			
Vehicle Volumes and	Adjustments			<u> </u>				
Major Street		Eastbound				Westbour	nd	
Movement	1	2	3		4	5 T		6 R
\(\langle \)	27	T 220	R	-	L	258		11
Volume (veh/h) Peak-Hour Factor, PHF	0.83	0.83	1.00		1.00	0.83		0.83
Hourly Flow Rate, HFR (veh/h)	32	265	0		0	310		13
Percent Heavy Vehicles	0		-		0			
Median Type			11	Undivi	ded		•	
RT Channelized			0					0
Lanes	0	1	0 0		1		0	
Configuration	LT						TI	
Upstream Signal		0	1	7		0		
Winor Street	+	Northbound	· · · · · · · · · · · · · · · · · · ·	*		Southbou	nd	
Viovement	7	8	9		10	11		12
NOVOITIONE	L	T	R		L	Т		R
Volume (veh/h)					6			35
Peak-Hour Factor, PHF	1.00	1.00	1.00		0.83	1.00		0.83
Hourly Flow Rate, HFR (veh/h)	0	0	0		7	0		42
Percent Heavy Vehicles	0	0	0		0	0		0
Percent Grade (%)		0				0		
Flared Approach		N				N		
Storage		0				0		
RT Channelized			0					0
Lanes	0	0	0		0	0		0
Configuration						LR		
Delay, Queue Length, a	nd Level of Ser	vice						
Approach	Eastbound	Westbound	1	Northbo	und	S	outhbound	k
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT						LR	
v (veh/h)	32						49	
C (m) (veh/h)	1248						663	
v/c	0.03						0.07	
95% queue length	0.08						0.24	
Control Delay (s/veh)	8.0						10.9	
LOS	Α						В	
Approach Delay (s/veh)				l			10.9	
Approach LOS						-	В	
		l					arated: 2/21/	2000 0

	T	WO-WAY STOP	CONTRO)L SUN	IMARY			
General Information			Site In	format	ion			
Analyst Agency/Co. Date Performed Analysis Time Period	DSK Kelly Engi 2/21/2022 AM Peak I		Interse Jurisdi Analys			Lockwood City of La 2025	l Cr. Rd. & Center	Sprice
Project Description Yea		ect						
East/West Street: Lockw					reet: Spruce	Ave.		
Intersection Orientation:			Study F	Period (h	nrs): 0.25			
Vehicle Volumes and	Adjustments							
Major Street		Eastbound	1 0	-		Westbou	nd T	
Movement	1 L	2 	3 R	-	4 L	5 T		6 R
Volume (veh/h)	27	232	K	-	L	309		11
Peak-Hour Factor, PHF	0.83	0.83	1.00		1.00	0.83		0.83
Hourly Flow Rate, HFR (veh/h)	32	279	0		0	372		13
Percent Heavy Vehicles	0				0			
Median Type		•		Undiv	ided	•		
RT Channelized			0					0
Lanes	0	1	0		0	1		0
Configuration	LT							TR
Upstream Signal		0				0		
Minor Street		Northbound		T		Southbou	ınd	
Movement	7	8	9		10	11		12
	L	Т	R		L	T		R
Volume (veh/h)					6			35
Peak-Hour Factor, PHF	1.00	1.00	1.00	!	0.83	1.00		0.83
Hourly Flow Rate, HFR (veh/h)	0	0	0		7	0		42
Percent Heavy Vehicles	0	0	0		0	0		0
Percent Grade (%)		0				0		***
Flared Approach		N				N		
Storage		0	1			0		
RT Channelized	-		0			1 -		0
Lanes	0	0	0	-	0	0		0
Configuration						LR		
Delay, Queue Length, a								
Approach	Eastbound	Westbound		Northbo			Southbound	
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT						LR	
v (veh/h)	32						49	
C (m) (veh/h)	1185						608	
v/c	0.03						0.08	
95% queue length	0.08						0.26	
Control Delay (s/veh)	8.1						11.4	
LOS	Α						В	
Approach Delay (s/veh)							11.4	
Approach LOS			В				В	

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General Information Analyst Agency/Co. Date Performed	DSK Kelly Engir 2/21/2022		Site Inf Intersect Jurisdict Analysis	ction ction	tion		Lockwood City of La 2022		Sprice	
Analysis Time Period	PM Peak I	Hour					4			
Project Description Exis			N (1 (0	11.01	1 0				-	
East/West Street: Lockw				North/South Street: Spruce Ave. Study Period (hrs): 0.25						
ntersection Orientation:			Study F	enou (i	115). 0.2	.5				
Vehicle Volumes and	Adjustments	Eastbound		- T			Westbour	nd		
Major Street Movement	1	2	3		4		5	10	6	
viovement	<u> </u>	T	R		L		T		R	
Volume (veh/h)	21	200					149		1	
Peak-Hour Factor, PHF	0.87	0.87	1.00		1.00)	0.87		0.87	
Hourly Flow Rate, HFR (veh/h)	24	229	0		0		171		1	
Percent Heavy Vehicles	0				0					
Median Type				Undiv	rided					
RT Channelized			0						0	
anes	0	1	0		0		1		0	
Configuration	LT								TR	
Jpstream Signal		0					0			
Minor Street		Northbound					Southbou	nd		
Movement	7	8	9		10)	11		12	
	L	T	R	-	L		T		R	
Volume (veh/h)	1.00	1.00	1.00 0.87		7	1.00		15 0.87		
Peak-Hour Factor, PHF	1.00	1.00	1.00 0.87					-		
Hourly Flow Rate, HFR (veh/h)	0	0	0		0		0		17 7	
Percent Heavy Vehicles	0	0	1 0				0			
Percent Grade (%)		0	T				l N			
Flared Approach		N 0					0			
Storage		0	0				0		0	
RT Channelized	0	0	0		0		0		0	
Lanes Configuration	-		-				LR			
	11 1 (0)									
Delay, Queue Length, a			,	Northbo	ound		Τ	Southbound	٠	
Approach	Eastbound	Westbound				0			12	
Movement	1	4	7	8		9	10	11	12	
Lane Configuration	LT							LR	-	
v (veh/h)	24							18		
C (m) (veh/h)	1417							834		
v/c	0.02							0.02		
95% queue length	0.05							0.07		
Control Delay (s/veh)	7.6							9.4		
LOS	Α							Α		
Approach Delay (s/veh)				•	•			9.4		
Approach LOS								A		

		WO-WAY STOP								
General Information			Site In	forma	tion					
Analyst Agency/Co. Date Performed Analysis Time Period	DSK Kelly Engi 2/21/2022 PM Peak l		Jurisdio	Intersection Jurisdiction Analysis Year			Lockwood Cr. Rd. & Sprice City of La Center 2025			
	ar 2025 w/o Proje	ect								
East/West Street: Lockw			North/South Street: Spruce Ave. Study Period (hrs): 0.25							
ntersection Orientation:	East-West		Study P	eriod ((hrs):	0.25				
Vehicle Volumes and	Adjustments									
Major Street		Eastbound				4	Westbour	nd	-	
Movement	1 L	2 	3 R			4 L	5 T		6 R	
Volume (veh/h)	40	261	K				198		3	
Peak-Hour Factor, PHF	0.87	0.87	1.00			1.00	0.87		0.87	
Hourly Flow Rate, HFR (veh/h)	45	299	0			0	227		3	
Percent Heavy Vehicles	0					0				
Median Type				Undi	vided					
RT Channelized			0						0	
Lanes	0	1	0			0	1		0	
Configuration	LT								TR	
Upstream Signal		0					0			
Minor Street		Northbound					Southbou	nd		
Movement	7	8	9			10	11		12	
	L	T	R				T		R	
Volume (veh/h)	1.00	1.00	1.00			3 0.87	1.00		27 0.87	
Peak-Hour Factor, PHF Hourly Flow Rate, HFR (veh/h)	0	0	0			3	0			
Percent Heavy Vehicles	0	0	0			0	0		7	
Percent Grade (%)	 	0					0			
Flared Approach		N					N			
Storage		0					0			
RT Channelized			0						0	
Lanes	0	0	0			0	0		0	
Configuration							LR			
Delay, Queue Length, a	nd Level of Serv	vice								
Approach	Eastbound	Westbound		Northb	ound		S	Southbound	1	
Movement	1	4	7	8		9	10	11	12	
Lane Configuration	LT							LR		
v (veh/h)	45							34		
C (m) (veh/h)	1350							746		
v/c	0.03							0.05		
95% queue length	0.10							0.14		
Control Delay (s/veh)	7.8							10.1		
LOS	Α							В		
Approach Delay (s/veh)								10.1		
Approach LOS								В		

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	TV	NO-WAY STOP	CONTRO	L SUN	/IMAKY				
General Information			Site In	forma	tion				
Analyst Agency/Co. Date Performed Analysis Time Period	DSK Kelly Engii 2/21/2022 PM Peak I		Interse Jurisdio Analysi	ction		Lockwood Cr. Rd. & Sprice City of La Center 2025			
Project Description Yea	ar 2025 with Proje	ect							
East/West Street: Lockw			North/S	outh St	reet: Spruce	e Ave.			
Intersection Orientation:	East-West		Study P	eriod (nrs): 0.25				
Vehicle Volumes and	Adjustments								
Major Street		Eastbound				Westbou	nd		
Movement	1	2	3		4	5		6	
	L	T	R		L	T		R	
Volume (veh/h)	40	299	1.00		1.00	221 0.87		3 0.87	
Peak-Hour Factor, PHF Hourly Flow Rate, HFR	0.87	0.87	1.00						
(veh/h)	45	343	0		0	254		3	
Percent Heavy Vehicles	0				0				
Median Type				Undiv	ided				
RT Channelized			0					0	
Lanes	0	1	0		0	1		0	
Configuration	LT							TR	
Upstream Signal		0				0			
Minor Street		Northbound				Southbou	ınd		
Movement	7	8	9		10	11		12	
	L	Т	R		L	T		R	
Volume (veh/h)	1.00	100	1.00		3	100		27	
Peak-Hour Factor, PHF Hourly Flow Rate, HFR	1.00	1.00	1.00		0.87	1.00		0.87	
(veh/h)	0	0	0		3	0		31	
Percent Heavy Vehicles	0	0	0 0		0		0		7
Percent Grade (%)		0				0			
Flared Approach		N				N			
Storage		0				0			
RT Channelized			0					0	
Lanes	0	0	0		0	0		0	
Configuration						LR			
Delay, Queue Length, a	nd Level of Serv	rice							
Approach	Eastbound	Westbound	1	Northbo	ound	8	Southbound		
Movement	1	4	7	8	9	10	11	1	
Lane Configuration	LT						LR		
v (veh/h)	45						34		
C (m) (veh/h)	1320						713		
v/c	0.03						0.05		
							0.15	1	
95% queue length	0.11						0.15		

7.8

Α

Control Delay (s/veh)

Approach Delay (s/veh)

Approach LOS

LOS

10.3 B

10.3

В

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0 11 7 4		WO-WAY STOP	Site Inf										
General Information			Site ini	ormat	ion		1	Crast Dd	9 John				
Analyst	DSK		Intersed	ction			Lockwood St.	Creek Ru.	& JOIII				
Agency/Co.	Kelly Engir	neering	Jurisdic	tion			City of La	Center					
Date Performed	2/21/2022		Analysi				2022						
Analysis Time Period	AM Peak I	Hour	7										
Project Description Exis													
East/West Street: Lockwo						John Sto	rm Ave.						
ntersection Orientation:	East-West		Study P	eriod (n	irs):	0.25							
Vehicle Volumes and	Adjustments	*					\A/ 11	- 1					
Major Street		Eastbound		-			Westbour	na	6				
Movement	1	2	3			4	5 T		R				
	L	T	R 20	-		7	148	_	IX				
Volume (veh/h)	0.87	93 0.82	0.82	-		0.82	0.82		0.87				
Peak-Hour Factor, PHF Hourly Flow Rate, HFR				-									
(veh/h)	0	113	24		8		180		0				
Percent Heavy Vehicles	0					0							
Median Type				Undiv	rided								
RT Channelized			0	0					0				
Lanes	0	1	0		0				0		1		0
Configuration			TR			LT							
Upstream Signal		0					0						
Minor Street		Northbound					Southbou	nd					
Movement	7	8	9			10	11		12				
	L	Т	R			L	T		R				
Volume (veh/h)	50		6			0.07	1.00		0.07				
Peak-Hour Factor, PHF	0.82	1.00	0.82	-	0.87		1.00		0.87				
Hourly Flow Rate, HFR (veh/h)	60	0	7		0		0		0		0		
Percent Heavy Vehicles	0	0	17		0		0		0		7		
Percent Grade (%)		0					0						
Flared Approach		N					N						
Storage		0							0				
RT Channelized			0						0				
Lanes	0	0	0			0	0		0				
Configuration		LR											
Delay, Queue Length, ar	nd Level of Serv	ice											
Approach	Eastbound	Westbound		Northbo	ound		5	Southboun	d				
Movement	1	4	7	8		9	10	11	12				
Lane Configuration		LT		LR									
v (veh/h)		8		67									
C (m) (veh/h)		1459		690)								
v/c		0.01		0.10									
95% queue length		0.02		0.32	2								
Control Delay (s/veh)		7.5		10.8									
LOS		A		B									
				10.8									
Approach Delay (s/veh)							+						
Approach LOS				D	В				/2022 6:				

0 11 6 4	•	WO-WAY STOP	Site Inf							
General Information			Site ini	ormat	ion		Laslavaad	Oranlı Dd	9 John	
Analyst	DSK		Intersed	ction			Lockwood St.	Creek Ra.	& JOIII	
Agency/Co.	Kelly Engi	neering	Jurisdic	tion			City of La	Center		
Date Performed	2/21/2022		Analysi				2025	Ocintor		
Analysis Time Period	AM Peak I	Hour	Allalysi	3 i cai			2020			
Project Description Yea	r 2025 w/o Proje	ot								
East/West Street: Lockwo		Ol .	North/Se	outh Str	reet:	John Sto	orm Ave.			
ntersection Orientation:			Study P							
Vehicle Volumes and										
Major Street		Eastbound		T			Westbound			
Movement	1	2	3			4	5		6	
	L	T	R			L	Т		R	
Volume (veh/h)		230	23			23	278			
Peak-Hour Factor, PHF	0.87	0.82	0.82			0.82	0.82		0.87	
Hourly Flow Rate, HFR (veh/h)	0	280	28		28		339		0	
Percent Heavy Vehicles	0					0				
Median Type		•		Undiv	ided					
RT Channelized			0	0				0		
Lanes	0	1	0			0	1		0	
Configuration			TR			LT				
Upstream Signal		0					0			
Minor Street		Northbound		T			Southbou	ınd		
Movement	7	8	9			10	11		12	
WIO VOITION.	L	Т	R			L	Т		R	
Volume (veh/h)	55		16							
Peak-Hour Factor, PHF	0.82	1.00	0.82		0.87		1.00		0.87	
Hourly Flow Rate, HFR (veh/h)	67	0	19			0	0		0	
Percent Heavy Vehicles	0	0	17			0	0		7	
Percent Grade (%)		0					0			
Flared Approach		N					N			
Storage		0					0			
RT Channelized			0						0	
Lanes	0	0	0			0	0		0	
Configuration		LR								
Delay, Queue Length, ar	nd Level of Serv	ice								
Approach	Eastbound	Westbound		Northbo	ound		3	Southboun	d	
Movement	1	4	7	8		9	10	11	12	
Lane Configuration		LT		LR						
		28		86			+		+	
v (veh/h)									+	
C (m) (veh/h)		1264		449					-	
v/c		0.02		0.19				-	-	
95% queue length		0.07		0.70						
Control Delay (s/veh)		7.9		14.9						
LOS		Α		В						
Approach Delay (s/veh)				14.9	9					
Approach LOS				В						
Apploacii LOS			L					orated: 2/21	/2022 6:1	

General Information			Site In	formati	on			
Analyst	DSK		Interse	ction			l Creek Rd	. & John
Agency/Co.	Kelly Engl	ineering	luriodi	otion		St. City of La	Contor	
Date Performed	2/21/2022		Jurisdi	is Year		2025	Center	
Analysis Time Period	AM Peak	Hour	Allalys	is real		2025		
Project Description Year	r 2025 with Proje	not.						
East/West Street: Lockw		7 01	North/S	outh Stre	eet: John St	form Ave		
ntersection Orientation:					rs): 0.25	01117140.		
Vehicle Volumes and				`				
Major Street		Eastbound			Westbou		nd	
Movement	1	2	3		4	5		6
	L	Т	R			T		R
/olume (veh/h)		241	23			309		
Peak-Hour Factor, PHF	0.87	0.82	0.82		0.82	0.82		0.87
Hourly Flow Rate, HFR veh/h)	0	293	28		28	376		0
Percent Heavy Vehicles	0		0					
Median Type				Undivid	ded			
RT Channelized			0	0				0
anes	0	1	0		0	1		0
Configuration			TR		LT			
Jpstream Signal		0				0		
Vinor Street		Northbound				Southbou	ınd	
Movement	7	8	9		10	11		12
	L	T	R		L	Т		R
/olume (veh/h)	55		16					
Peak-Hour Factor, PHF	0.82	1.00	0.82		0.87	1.00		0.87
Hourly Flow Rate, HFR (veh/h)	67	0	19		0	0		0
Percent Heavy Vehicles	0	0	17		0	0		7
Percent Grade (%)		0				0		
Flared Approach		N				N		
Storage		0				0		
RT Channelized		y	0					0
_anes	0	0	0		0	0		0
Configuration		LR						
Delay, Queue Length, an	d Level of Serv	ice						
Approach	Eastbound	Westbound		Northbou	ınd	3	Southbound	d
Movement	1	4	7	8	9	10	11	12
Lane Configuration	-	LT		LR	+	+		+
v (veh/h)		28		86				+
C (m) (veh/h)		1250		422		+		+
				0.20		-		-
I/C		0.02				-		+
95% queue length		0.07		0.75				+
Control Delay (s/veh)		7.9		15.7				-
LOS		Α		С				
Approach Delay (s/veh)				15.7				
Approach LOS				С				

		WO-WAY STOP								
General Information			Site In	tormat	ion					
Analyst	DSK		Interse	ction			Lockwood St.	Creek Ro	l. & John	
Agency/Co.	Kelly Engil	neering	Jurisdio	otion			City of La	Contor		
Date Performed	2/21/2022		Analysi				2022	Center		
Analysis Time Period	PM Peak I	Hour	Allalysi	is i eai			2022			
Desiret Description - Frie	ation or									
Project Description Existing Existing Exist Exis			North/S	outh Str	reet: Joh	n Sto	rm Ave			
ntersection Orientation:					rs): 0.25		1111 7 (0 .			
Vehicle Volumes and				,						
Major Street	Adjustificitis	Eastbound		T			Westbound			
Movement	1	2	3		4		5		6	
viovomone	i	T	R		L		Т		R	
/olume (veh/h)		186	11							
Peak-Hour Factor, PHF	0.88	0.88	0.88		0.88		0.88		0.88	
Hourly Flow Rate, HFR veh/h)	0	211	12		11		170		0	
Percent Heavy Vehicles	0	-	-		0					
Median Type		•		Undiv	ided					
RT Channelized			0						0	
_anes	0	1	0		0		1		0	
Configuration			TR		LT					
Jpstream Signal		0					0			
Minor Street		Northbound		T			Southbou	nd		
Movement	7	8	9		10		11		12	
	L	Т	R		L		Т		R	
Volume (veh/h)	37		24							
Peak-Hour Factor, PHF	0.88	0.88	0.88		0.88		0.88		0.88	
Hourly Flow Rate, HFR (veh/h)	42	0	27		0		0		0	
Percent Heavy Vehicles	3	0	0		0		0		7	
Percent Grade (%)		0					0			
Flared Approach		N					N			
Storage		0					0			
RT Channelized			0						0	
Lanes	0	0	0		0		0		0	
Configuration		LR								
Delay, Queue Length, ar	nd Level of Serv	ice								
Approach	Eastbound	Westbound		Northbo	ound		S	Southboun	d	
Movement	1	4	7	8		9	10	11	12	
Lane Configuration		LT		LR						
v (veh/h)		11		69						
C (m) (veh/h)		1358		666						
V/C		0.01		0.10						
95% queue length		0.02		0.35	_		1		1	
Control Delay (s/veh)		7.7		11.0						
				B			+		-	
LOS		A					+			
Approach Delay (s/veh)				11.0			-			
Approach LOS				В						

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	Т	WO-WAY STOP	CONTRO	L SUI	MMA	RY			
General Information			Site In	forma	tion				
Analyst Agency/Co. Date Performed Analysis Time Period	DSK Kelly Engi 2/21/2022 PM Peak	?	Interse Jurisdio Analys	ction			Lockwood St. City of La 2025		d. & John
Project Description Yea	ar 2025 w/o Proje	ect							
East/West Street: Lockw			North/S	outh S	treet:	John Sto	rm Ave.		
Intersection Orientation:	East-West		Study F	eriod ((hrs):	0.25			
Vehicle Volumes and	Adjustments								
Major Street		Eastbound					Westbou	nd	
Movement	1	2	3			4	5		6
Volume (veh/h)	L	T 261	12			15	T 206		R
Peak-Hour Factor, PHF	0.88	0.88	0.88			0.88	0.88		0.88
Hourly Flow Rate, HFR (veh/h)	0	296	13			17	234		0
Percent Heavy Vehicles	0				0				
Median Type		*	Undivided						
RT Channelized			0						0
Lanes	0	1	0			0	1		0
Configuration			TR			LT			
Upstream Signal		0					0		
Minor Street	T	Northbound					Southbou	ınd	
Movement	7	8	9			10	11		12
	L	Т	R			L	T		R
Volume (veh/h)	39		28						
Peak-Hour Factor, PHF	0.88	0.88	0.88		0.88		0.88		0.88
Hourly Flow Rate, HFR (veh/h)	44	0	31		0		0		0
Percent Heavy Vehicles	3	0	0			0 0			7
Percent Grade (%)		0					0		
Flared Approach		N					N		
Storage		0					0		
RT Channelized			0						0
Lanes	0	0	0			0	0		0
Configuration		LR							
Delay, Queue Length, ar	T T	T T					T		
Approach	Eastbound	Westbound		Northb	ound			Southbour	nd
Movement	1	4	7	8		9	10	11	12
Lane Configuration		LT		LF					
v (veh/h)		17		75	j				
C (m) (veh/h)		1263		558	8				
v/c		0.01		0.1	3				
95% queue length		0.04		0.4	6				
Control Delay (s/veh)		7.9		12.					
LOS		Α		В					
Approach Delay (s/veh)		-		12.		L			
Approach LOS				В		************			
Approach LOG			В		1				

	Т	WO-WAY STOP	CONTRO	LSUN	/MA	KY					
General Information			Site Inf	ormat	tion						
Analyst Agency/Co. Date Performed Analysis Time Period	DSK Kelly Engii 2/21/2022 PM Peak I		Intersed Jurisdic Analysi	tion			Lockwood St. City of La 2025		d. & John		
Project Description Yea	r 2025 with Proje	ct									
East/West Street: Lockw		01	North/So	outh St	reet:	John Sto	rm Ave.				
Intersection Orientation:	East-West		Study P	eriod (l	hrs):	0.25					
Vehicle Volumes and	Adjustments										
Major Street		Eastbound					Westbound				
Movement	1	2	3			4	5		6		
	L	T 207	R 12			15	T 228	_	R		
Volume (veh/h) Peak-Hour Factor, PHF	0.88	297 0.88					0.88		0.88		
Hourly Flow Rate, HFR (veh/h)	0.88	337	0.88 0.88 13 17		259		0				
Percent Heavy Vehicles	0					0					
Median Type		1	Undivided								
RT Channelized			0				0				
Lanes	0	1	0			0	1		0		
Configuration	-	'	TR			LT					
Upstream Signal		0	1				0				
Minor Street		Northbound					Southbou	ınd			
Movement	7	8	9			10	11		12		
WOVERNOTE	L	T	R			L	Т		R		
Volume (veh/h)	39		28								
Peak-Hour Factor, PHF	0.88	0.88	0.88			0.88	0.88		0.88		
Hourly Flow Rate, HFR (veh/h)	44	0	31			0	0		0		
Percent Heavy Vehicles	3	0	0		0		0		0		7
Percent Grade (%)		0					0				
Flared Approach		N					N				
Storage		0					0				
RT Channelized			0						0		
Lanes	0	0	0			0	0		0		
Configuration		LR									
Delay, Queue Length, a			I				Т .				
Approach	Eastbound	Westbound		Northb	ound			Southbou	1		
Movement	1	4	7	8		9	10	11	12		
Lane Configuration		LT		LF	?						
v (veh/h)		17		75	5						
C (m) (veh/h)		1220		510	6						
v/c		0.01		0.1	5						
95% queue length		0.04		0.5							
		8.0		13.			1				
Control Delay (s/veh)				-			+				
LOS		Α	-	B			+				
Approach Delay (s/veh)		-		13.			-				
Approach LOS				В							

	Т	WO-WAY STOP	CONTRO	L SUM	MMA	RY			
General Information			Site In	forma	tion				
Analyst	DSK		Interse				Lockwood		£ vy Ave.
Agency/Co.	Kelly Engl		Jurisdi				City of La	Center	
Date Performed	2/21/2022		Analys	is Year			2022		
Analysis Time Period	AM Peak	Hour							
	sting		1						
East/West Street: Lockw		d				Ivy Ave.			
Intersection Orientation:			Study F	erioa (nrs):	0.25			
Vehicle Volumes and	Adjustments) A / _ / I		
Major Street	1	Eastbound	3			4	Westbour 5	na	6
Movement			R			4	T		R
Volume (veh/h)	147	132	15	-		10	246		39
Peak-Hour Factor, PHF	0.72	0.72	0.72			0.72	0.72		0.72
Hourly Flow Rate, HFR (veh/h)	204	183	20			13	341		54
Percent Heavy Vehicles	5		-			0			
Median Type		•		Undiv	rided				
RT Channelized			0						0
Lanes	1	1	0			1	1		0
Configuration	L		TR			L			TR
Upstream Signal		0					0		
Minor Street		Northbound					Southbou	nd	
Movement	7	8	9			10	11		12
	L	Т	R			L	Т		R
Volume (veh/h)	31	1	19			10			214
Peak-Hour Factor, PHF	0.72	0.72	0.72			0.72	0.72		0.72
Hourly Flow Rate, HFR (veh/h)	43	1	26			13	4		297
Percent Heavy Vehicles	55	0	5			10	0		7
Percent Grade (%)		0					0		
Flared Approach		N					N		
Storage		0					0		
RT Channelized			0						0
Lanes	1	1	0			1	1		0
Configuration	L		TR			L			TR
Delay, Queue Length, a									
Approach	Eastbound	Westbound		Northbo	ound		S	outhboun	d
Movement	1	4	7	8		9	10	11	12
Lane Configuration	L	L	L			TR	L		TR
v (veh/h)	204	13	43			27	13		301
C (m) (veh/h)	1147	1381	66			749	175		646
v/c	0.18	0.01	0.65			0.04	0.07		0.47
95% queue length	0.65	0.03	2.83			0.11	0.24		2.48
Control Delay (s/veh)	8.8	7.6	129.9			10.0	27.2		15.3
LOS	Α	Α	F			Α	D		С
Approach Delay (s/veh)				83.7	7			15.8	
Approach LOS				F			С		
ipprodon LOO		1				1			

			0:4							
Analyst Agency/Co. Date Performed Analysis Time Period	DSK Kelly Engi 2/21/2022 PM Peak		Interse Jurisdid Analys	ction	on	Lockwood City of La 2022		Evy Ave		
Project Description Exis	ting									
East/West Street: Lockwo		1			eet: Ivy Ave.					
Intersection Orientation:	East-West		Study F	eriod (hr	rs): 0.25					
Vehicle Volumes and	Adjustments									
Major Street		Eastbound			4	Westbour	nd			
Movement	1	2	3	3 R		5 T		6		
	L	T		2 R		144	15			
Volume (veh/h)	179 0.90	289 0.90	0.90		0.90	0.90		0.90		
Peak-Hour Factor, PHF Hourly Flow Rate, HFR	198	321	2		1	160		16		
(veh/h)	+				0					
Percent Heavy Vehicles	0			I la alissia						
Median Type		1	Undivided 0							
RT Channelized			0			1				
Lanes	1	1	0		1	1				
Configuration	L		TR		L			TR		
Upstream Signal		0				0				
Minor Street		Northbound				Southbou	nd	40		
Movement	7	8	9		10	11		12 R		
	L	T	R		L	T 2		132		
Volume (veh/h)	23	0.90	0.90		0.90	0.90		0.90		
Peak-Hour Factor, PHF Hourly Flow Rate, HFR	0.90				b-1					
(veh/h)	25	6	20		14 0	0		146 		
Percent Heavy Vehicles	0		0			0				
Percent Grade (%)		0				l N				
Flared Approach		N								
Storage		0				0		0		
RT Channelized			0	-		1		0		
Lanes	1	1	0		1	1				
Configuration	L		TR		L			TR		
Delay, Queue Length, ar	nd Level of Ser	T								
Approach	Eastbound	Westbound		Northbou	und	8	Southbound			
Movement	1	4	7	8	9	10	11	12		
Lane Configuration	L	L	L		TR	L		TR		
v (veh/h)	198	1	25		26	14		148		
C (m) (veh/h)	1412	1248	175		496	222		851		
v/c	0.14	0.00	0.14		0.05	0.06		0.17		
95% queue length	0.49	0.00	0.49		0.17	0.20		0.63		
Control Delay (s/veh)	8.0	7.9	29.0		12.7	22.3		10.1		
LOS	Α	Α	D		В	С		В		
Approach Delay (s/veh)				20.7			11.2			
Approach LOS			С		В					

	Т	WO-WAY STOR	CONTRO	DL SUI	VIM/	ARY			
General Information			Site In	forma	tion			ē	
Analyst Agency/Co. Date Performed Analysis Time Period	DSK Kelly Eng 2/21/2022 PM Peak	? Hour	Interse Jurisdi Analys				Lockwood City of La 2025	l Cr. Rd. & Center	E vy Ave.
	ar 2025 w/o Proj								
East/West Street: Lockw		d				: Ivy Ave.			
Intersection Orientation:	East-West		Study F	Period (hrs):	0.25			
Vehicle Volumes and	Adjustments								
Major Street		Eastbound	T 0				Westbou	nd	
Movement	1 L		3 R			4 	5 T		6 R
Volume (veh/h)	189	366	2			0	194		23
Peak-Hour Factor, PHF	0.90	0.90	0.90	,		0.90	0.90	-+	0.90
Hourly Flow Rate, HFR (veh/h)	210	406	2			0	215		25
Percent Heavy Vehicles	0					0			
Median Type				Undiv	videa	1			
RT Channelized			0						0
Lanes	1	1	0			1	1		0
Configuration	L		TR			L			TR
Upstream Signal		0					0		
Minor Street	T	Northbound					Southbou	ınd	
Movement	7	8	9			10	11		12
	L	Т	R			L	T		R
Volume (veh/h)	19	6	15			6	1		139
Peak-Hour Factor, PHF Hourly Flow Rate, HFR	0.90	0.90	0.90			0.90	0.90		0.90
(veh/h)	21	6	16			6 0	0		154 0
Percent Heavy Vehicles	- 0		0			U	0		0
Percent Grade (%)		0	1						
Flared Approach		N	-				N		
Storage		0	0				0		0
RT Channelized	1	1	0			1	1	-	0
Lanes Configuration	<u> </u>		TR			L	· '		TR
Delay, Queue Length, a		vice				_			
				Northbo	nund			outhbound	ı
Approach	Eastbound	Westbound			Juild				
Movement	1 <i>L</i>	4	7	8		9 TR	10 <i>L</i>	11	12 TR
Lane Configuration		L	L 21						
v (veh/h)	210	0	21	-	-	22	6		155
C (m) (veh/h)	1339	1162	129			390	169		799
v/c	0.16	0.00	0.16	-		0.06	0.04		0.19
95% queue length	0.56	0.00	0.56	-		0.18	0.11		0.72
Control Delay (s/veh)	8.2	8.1	38.3			14.8	27.1		10.6
LOS	Α	Α	Е			В	D	44.0	В
Approach Delay (s/veh)				26.2				11.2	
Approach LOS			D			В			

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2 Llf			Site In	format	tion				
Analyst Agency/Co. Date Performed Analysis Time Period	DSK Kelly Engii 2/21/2022 PM Peak I		Intersed Jurisdic Analysi	ction	uon		Lockwood City of La 2025		Evy Ave
Project Description Yea	r 2025 with Proje	ect							
East/West Street: Lockw			North/S	outh St	reet:	Ivy Ave.			
ntersection Orientation:	East-West		Study P	eriod (ł	nrs):	0.25			
Vehicle Volumes and	Adjustments								
Major Street		Eastbound					Westbour	nd	
Movement	1	2	3			4	5		6
	L	T	R 2			0 0	T 214		23
/olume (veh/h)	0.90	0.90	0.90	-		0.90	0.90		23 0.90
Peak-Hour Factor, PHF Hourly Flow Rate, HFR (veh/h)	210	442	2			0	237		25
Percent Heavy Vehicles	0					0			
Median Type				Undiv	rided				
RT Channelized			0						0
anes	1	1	0			1	1	1 (
Configuration	L		TR			L			TR
Jpstream Signal		0					0		
Minor Street		Northbound					Southbou	nd	
Movement	7	8	9			10	11		12
	L	Т	R			L	Т		R
Volume (veh/h)	19	6	15			6	1		139
Peak-Hour Factor, PHF	0.90	0.90	0.90			0.90	0.90		0.90
Hourly Flow Rate, HFR (veh/h)	21	6	16			6	1		154 0
Percent Heavy Vehicles	0	0	0			0	0		
Percent Grade (%)		0					0		
Flared Approach		N					N		
Storage		0					0		0
RT Channelized			0			1	1		0
Lanes	1	1	0			1	'		TR
Configuration	L		TR			L			111
Delay, Queue Length, a			ı				T 6		
Approach	Eastbound	Westbound		Northbo			-	outhbound	
Movement	1	4	7	8		9	10	11	12
Lane Configuration	L	L	L			TR	L		TR
v (veh/h)	210	0	21			22	6		155
C (m) (veh/h)	1314	1127	117			365	153		776
v/c	0.16	0.00	0.18			0.06	0.04		0.20
95% queue length	0.57	0.00	0.62			0.19	0.12		0.74
Control Delay (s/veh)	8.3	8.2	42.4			15.5	29.5		10.8
LOS	A A	A	E			C	D		В
			-	28.0	 6			11.5	
Approach Delay (s/veh)								B	
Approach LOS				D				B erated: 2/21/3	

General Information			Site Info	ormati	on					
Analyst Agency/Co. Date Performed Analysis Time Period	DSK Kelly Engi 2/21/2022 AM Peak		Intersec Jurisdict Analysis	tion tion	011	E. 4th St. & Aspen Ave. City of La Center 2022				
Project Description Exis	sting									
East/West Street: E 4th					eet: <i>Aspen A</i>	ve.				
ntersection Orientation:	East-West		Study Pe	eriod (hi	rs): 0.25					
Vehicle Volumes and	Adjustments									
Major Street		Eastbound				Westboun	d			
Movement	1 .	2	3		4	5 T		6 R		
	L 45	T 297	R		L	425		16		
Volume (veh/h) Peak-Hour Factor, PHF	45 0.81	0.81	1.00		1.00	0.81		0.81		
Hourly Flow Rate, HFR (veh/h)	55	366	0		0	524		19		
Percent Heavy Vehicles	7				0					
Median Type				Undivid	ded					
RT Channelized			0					0		
Lanes	1	1	0		0	1		0		
Configuration	L	T						TR		
Upstream Signal		0				0				
Minor Street		Northbound				Southbou	nd			
Movement	7	8	9		10	11				
	L	Т	R		L	Т				
Volume (veh/h)					47	1.00		124		
Peak-Hour Factor, PHF	1.00	1.00	1.00		0.81	1.00		1.00		
Hourly Flow Rate, HFR (veh/h)	0	0	0		58	0		124 0		
Percent Heavy Vehicles	0	0	0		4					
Percent Grade (%)		0	1			0				
Flared Approach		N	-			N 0				
Storage		0	0			-		0		
RT Channelized	0	0	0		1	0		1		
Lanes	- 0	-	-		L	-		 R		
Configuration										
Delay, Queue Length, a				Vorthboi	und		outhbound	1		
Approach	Eastbound	Westbound								
Movement	1	4	7	8	9	10	11	12		
Lane Configuration	L					L		R		
v (veh/h)	55					58		124		
C (m) (veh/h)	1001					249		550		
v/c	0.05					0.23		0.23		
95% queue length	0.17					0.88		0.86		
Control Delay (s/veh)	8.8					23.8		13.4		
LOS	A					С		В		
							16.7			
Approach Delay (s/veh)						1	C			
Approach LOS				CS+TM V			ated: 2/21/20			

	T\	WO-WAY STOP	CONTRO	L SUI	MMA	RY			
General Information			Site In	forma	tion				
Analyst Agency/Co. Date Performed Analysis Time Period	DSK Kelly Engli 2/21/2022 AM Peak I		Interse Jurisdio Analysi	ction			E. 4th St. 6 City of La 2025	-	e.
	ar 2025 w/o Proje	ect							
East/West Street: E 4th S	St.					Aspen A	ve.		
Intersection Orientation:	East-West		Study P	eriod ((hrs):	0.25			
Vehicle Volumes and	Adjustments								
Major Street		Eastbound	1 0				Westbour	nd	6
Movement	1	2 	3 R			4 	5 T		6 R
Volume (veh/h)	48	325	N				481		19
Peak-Hour Factor, PHF	0.81	0.81	1.00			1.00	0.81	(0.81
Hourly Flow Rate, HFR (veh/h)	59	401	0			0	593		23
Percent Heavy Vehicles	7					0			
Median Type			_	Undi	vided		,		
RT Channelized			0						0
Lanes	1	1	0			0	1		0
Configuration	L	T							TR
Upstream Signal		0					0		
Minor Street		Northbound					Southbou	nd	
Movement	7	8	9			10	11		12
	L	T	R			51	Т		R 131
Volume (veh/h) Peak-Hour Factor, PHF	1.00	1.00	1.00			0.81	1.00		1.00
Hourly Flow Rate, HFR (veh/h)	0	0	0			62	0		131
Percent Heavy Vehicles	0	0	0			4	0		0
Percent Grade (%)		0					0		
Flared Approach		N					N		
Storage		0					0		
RT Channelized			0						0
Lanes	0	0	0			1	0		1
Configuration						L			R
Delay, Queue Length, a	nd Level of Sen	vice							
Approach	Eastbound	Westbound		Northb	ound		S	outhbound	
Movement	1	4	7	8	3	9	10	11	12
Lane Configuration	L						L		R
v (veh/h)	59						62		131
C (m) (veh/h)	940						211		502
v/c	0.06						0.29		0.26
95% queue length	0.20						1.17		1.04
Control Delay (s/veh)	9.1						29.0		14.7
LOS	Α						D		В
Approach Delay (s/veh)								19.3	
Approach LOS				***************************************				С	
Approach LOS									

			CONTROL						
General Information			Site Info		on				
Analyst Agency/Co. Date Performed Analysis Time Period	DSK Kelly Engii 2/21/2022 AM Peak I		Jurisdict Analysis	tion		E. 4th St. & City of La (2025		ve.	
Project Description Yea	r 2025 with Proje	ect							
East/West Street: E 4th S			North/So	outh Stre	eet: Aspen A	ve.			
Intersection Orientation:	East-West		Study Pe	eriod (hr	rs): 0.25				
Vehicle Volumes and	Adjustments								
Major Street		Eastbound				Westboun	d		
Movement	1	2	3		4	5		6	
	L	T	R		L	T 506		R 21	
Volume (veh/h)	48 0.81	333 0.81	1.00		1.00	506 0.81		0.81	
Peak-Hour Factor, PHF Hourly Flow Rate, HFR (veh/h)	59	411	0		0	624		25	
Percent Heavy Vehicles	7				0				
Median Type			1	Undivid	ded				
RT Channelized			0					0	
Lanes	1	1	0		0	1		0	
Configuration	L	T						TR	
Upstream Signal		0				0			
Minor Street		Northbound				Southbour	nd		
Movement	7	8	9		10	11		12	
,	L	Т	R		L	T		R	
Volume (veh/h)					52			131	
Peak-Hour Factor, PHF	1.00	1.00	1.00		0.81	1.00		1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0		64	0		131	
Percent Heavy Vehicles	0	0	0		4	0			
Percent Grade (%)		0	_			0			
Flared Approach		N	-			N			
Storage		0				0	_	0	
RT Channelized		0	0		1	0		1	
Lanes	0		1		L	"	-	 R	
Configuration					L			- / \	
Delay, Queue Length, ar						Τ .	outhbound	1	
Approach	Eastbound	Westbound		lorthbou					
Movement	1	4	7	8	9	10	11	12	
Lane Configuration	L					L		R	
v (veh/h)	59					64		131	
C (m) (veh/h)	914					199		481	
v/c	0.06					0.32		0.27	
95% queue length	0.21					1.32		1.10	
Control Delay (s/veh)	9.2					31.4		15.3	
LOS	Α					D		С	
Approach Delay (s/veh)							20.6		
, , , , , , , , , , , , , , , , , , , ,									

	T\	WO-WAY STOP	CONTRO	L SUI	MMA	RY			
General Information			Site In	forma	tion				
Analyst Agency/Co. Date Performed Analysis Time Period	DSK Kelly Engli 2/21/2022 PM Peak I		Intersed Jurisdic Analysi	ction			E. 4th St. 6 City of La 2022		/e.
	sting								
East/West Street: E 4th S						Aspen A	ve.		
Intersection Orientation:	East-West		Study P	eriod (hrs):	0.25			
Vehicle Volumes and	Adjustments								
Major Street		Eastbound					Westbour	nd	
Movement	1	2	3 R			 L	5 T		6 R
\(\langle \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	171	425	R			L	229	_	11
Volume (veh/h) Peak-Hour Factor, PHF	0.93	0.93	1.00			1.00	0.93		0.93
Hourly Flow Rate, HFR (veh/h)	183	456	0			0	246		11
Percent Heavy Vehicles	0					0			
Median Type				Undi	vided			'	
RT Channelized			0						0
Lanes	1	1	0			0	1		0
Configuration	L	T							TR
Upstream Signal		0					0		
Minor Street	—	Northbound					Southbou	nd	
Movement	7	8	9			10	11		12
	L	Т	R			L	Т		R
Volume (veh/h)						11			87
Peak-Hour Factor, PHF	1.00	1.00	1.00			0.93	1.00		0.93
Hourly Flow Rate, HFR (veh/h)	0	0	0			11	0		93
Percent Heavy Vehicles	0	0	0			0	0		0
Percent Grade (%)		0					0		
Flared Approach		N					N		
Storage		0					0		
RT Channelized			0						0
Lanes	0	0	0			1	0		1
Configuration						L			R
Delay, Queue Length, a	nd Level of Sen	/ice					_		
Approach	Eastbound	Westbound		Northb	ound		S	outhbound	1
Movement	1	4	7	8		9	10	11	12
Lane Configuration	L						L		R
v (veh/h)	183						11		93
C (m) (veh/h)	1320						212		792
v/c	0.14						0.05		0.12
95% queue length	0.48						0.16		0.40
Control Delay (s/veh)	8.2						22.9		10.1
LOS	Α						C		В
Approach Delay (s/veh)								11.5 B	
Approach LOS								Ь	

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General Information			Site Inf	formati	ion			
Analyst Agency/Co. Date Performed Analysis Time Period	DSK Kelly Engi 2/21/2022 PM Peak		Intersed Jurisdic Analysi	ction	on	E. 4th St. 8 City of La (2025		ve.
Project Description Yea	r 2025 w/o Proje	ect						
East/West Street: E 4th S	St.				eet: <i>Aspen A</i>	ve.		
Intersection Orientation:	East-West		Study P	eriod (h	rs): 0.25			
Vehicle Volumes and	Adjustments							
Major Street		Eastbound				Westboun	d	
Movement	11	2	3		4	5 T		6
\(\frac{1}{2} \\ \frac{1}{2} \\ \fra	181	T 483	R		L	263		R 13
Volume (veh/h) Peak-Hour Factor, PHF	0.93	0.93	1.00		1.00	0.93		0.93
Hourly Flow Rate, HFR (veh/h)	194	519	0		0	282		13
Percent Heavy Vehicles	0				0			
Median Type				Undivid	ded			
RT Channelized			0			T		0
Lanes	1	1	0		0	1		0
Configuration	L	T						TR
Upstream Signal		0				0		
Minor Street		Northbound				Southbour	nd	
Movement	7	8	9		10	11		12
	L	Т	R		L	Т		R
Volume (veh/h)					14			92
Peak-Hour Factor, PHF	1.00	1.00	1.00		0.93	1.00		0.93
Hourly Flow Rate, HFR (veh/h)	0	0	0		15	0		98
Percent Heavy Vehicles	0	0	0		0	0		0
Percent Grade (%)		0	_			0		
Flared Approach		N				N		
Storage		0	<u> </u>			0		
RT Channelized			0				_	0
Lanes	0	0	0		1	0		1
Configuration					L			R
Delay, Queue Length, ar						Τ -		
Approach	Eastbound	Westbound		Northbou			outhbound	_
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L					L		R
v (veh/h)	194					15		98
C (m) (veh/h)	1278					176		756
v/c	0.15					0.09		0.13
95% queue length	0.53					0.28		0.44
	8.3					27.4		10.5
Control Delay (s/veh)						D D		B
LOS	A					D	40.7	
Approach Delay (s/veh)					***************************************		12.7	
Approach LOS							В	

	T\	WO-WAY STOP	CONTRO	L SU	MMA	RY			
General Information			Site In	forma	tion				
Analyst Agency/Co. Date Performed Analysis Time Period	DSK Kelly Engi 2/21/2022 PM Peak I	Hour	Interse Jurisdio Analys	ction	r		E. 4th St. 6 City of La 2025	-	Ave.
	ar 2025 with Proj	ect							
East/West Street: E 4th 3						: Aspen A	ve.		
Intersection Orientation:	East-West		Study F	eriod ((hrs):	0.25			
Vehicle Volumes and	Adjustments								
Major Street		Eastbound					Westbour	nd	
Movement	1	2	3			4	5		6
	L	T	R			L	T		R
Volume (veh/h)	181	513	1.00			1.00	282 0.93		14 0.93
Peak-Hour Factor, PHF Hourly Flow Rate, HFR	0.93	0.93 551	1.00			0	303		15
(veh/h) Percent Heavy Vehicles	0					0			
	-			Undi	vided				
Median Type RT Channelized		T	T 0	Orial	viaca				0
	1	1	0			0	1		0
Lanes	L	T					<u> </u>		TR
Configuration		0					0		
Upstream Signal							Southbou	nd	
Minor Street	7	Northbound 8	9			10	11	iiu	12
Movement	/	, ° T	R			L	T T		R
Volume (veh/h)		-	IX			16	 		92
Peak-Hour Factor, PHF	1.00	1.00	1.00			0.93	1.00		0.93
Hourly Flow Rate, HFR (veh/h)	0	0	0			17	0		98
Percent Heavy Vehicles	0	0	0			0	0		0
Percent Grade (%)		0					0	•	
Flared Approach		N					N		
Storage		0					0		
RT Channelized			0						0
Lanes	0	0	0			1	0		1
Configuration						L			R
Delay, Queue Length, ar	nd Level of Sen	vice							
Approach	Eastbound	Westbound		Northb	ound		S	outhbou	nd
Movement	1	4	7	8		9	10	11	12
Lane Configuration	L	-T	,	<u> </u>		-	L		R
v (veh/h)	194						17		98
<u> </u>	1253						163		735
C (m) (veh/h)	0.15						0.10		0.13
95% queue length	0.75						0.34		0.46
Control Delay (s/veh)	8.4						29.6		10.7
LOS	A						D		В
Approach Delay (s/veh)				L		I		13.5	
Approach LOS							†	В	
Apploacificos									

					ROU	NDABO	UT REP	ORT								
General Information							Site Inf	orm	atio	n						
Analyst DSK Agency or Co. Kelly E Date Performed 2/21/20 Time Period AM Pe Peak Hour Factor 0.87	022						Intersect E/W Stree N/S Stree Analysis Project	tion eet N eet Na Yea	ame	Paci W 4t		& 4th St.				
Project Description:																
Volume Adjustment an	d Site	Chara	cteris	tics												
		Е	3			V	/B				NB				SB	
	L	Т	R	Û	L	Т	R	U	L	. T	R	U	L	Т	R	U
Number of Lanes (N)	0	0	0		0	0	0		0	1	0		0	1	0	
Lane Assignment							LR					Γ				LT
Right-Turn Bypass		No	пе			No	ne			Non	-Yielding			Ν	one	
Conflicting Lanes		1					1				1				1	
Volume (V), veh/h				0	473		52	0		67		0	46	179		0
Heavy Veh. Adj. (f _{HV}), %	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3
Pedestrians Crossing		0)				0				0	
Critical and Follow-Up	Heady			nent												
Chilcal and I ollow-op	readi	Tuy Ac	El				WB		П		NB				SB	
		Left	Rig		Bypass	Left	Right	Вур	200	Left	Right	Bypass	Lef	t F	Right	Bypass
Critical Headway (sec)		5.192	_	_	5.1929		5.1929	-	-	5.1929	5.1929	5.1929	5.192	_	1929	5.1929
		3.185		-			3.1858	-	-	3.1858	-	3.1858	3.18	_		3.1858
Follow-Up Headway (sec)		3.1036	3.10	000	3.1000	3.1000	3.7000	3.70	,50	3.1000	3.7000	3.7000	3.70	0 3.	7000	3.7000
Flow Computations		Π		<u> </u>		l	WB		\neg		NB				SB	
		1 . 0	E		D	1 - 64		D	_	1.4		Dungag	106			Bungan
Oine leties Floor (M) as the		Left	Rig		Bypass	Left	Right	Вур	ass	Left	Right	Bypass	Lef		Right	Bypass
Circulating Flow (V _c), pc/h		-	82				79		\dashv		54		-		560	
Exiting Flow (V _{ex}), pc/h			54	1			0	_	_		141	T	-		772	Τ
Entry Flow (V _e), pc/h				_			622	_	_		79	701	-		266	
Entry Volume veh/h							604				77	681	<u> </u>		258	
Capacity and v/c Ratio	s															
			E				WB		_		NB		_		SB	Т
		Left	Rig	ght	Bypass	Left	Right	Вур	ass	Left	Right	Bypass	Lef	\rightarrow	Right	Bypass
Capacity (c _{PCE}), pc/h			0				1044				1070				645	
Capacity (c), veh/h			0				1013				1039				627	
v/c Ratio (X)							0.60				0.07				0.41	
Delay and Level of Ser	vice															
			Е	В			WB				NB				SB	
		Left	Rig	ght	Bypass	Left	Right	Вур	ass	Left	Right	Bypass	Lef	t I	Right	Bypass
Lane Control Delay (d), s/v	eh			\neg			11.6				4.1	0.0			11.8	
Lane LOS			F	:			В				A				В	
Lane 95% Queue				\neg			4.1				0.2				2.0	
Approach Delay, s/veh							11.64	1			0.42			1	11.76	
Approach LOS, s/veh		†					В				A				В	
Intersection Delay, s/veh		 			· · · · · · · · · · · · · · · · · · ·	L			6.4	41						
Intersection LOS		-														
LING SCOUNT LOO										101						

Critical Headway (sec)						ROU	INDABO	UT REP	ORT								
Intersection Pacific Hyely, & 4th St. Each Street Name Pacific Hyely, & 4th St. Each Street Name Pacific Hyely, & 4th St. Each Street Name Pacific Hyely, & 4th St.	General Information							Site In:	form	atio	n						
Volume Adjustment and Site Characteristics	Analyst DSK Agency or Co. Kelly E Date Performed 2/21/2 Time Period AM Pe Peak Hour Factor 0.87	022						Intersect E/W Str N/S Stre Analysis	eet Neet Na eet Na eet Na	ame	Pacin W 4t Pacin 2025	h St. fic Hwy.		t			
Number of Lanes (N)		10"	01	, .													
Number of Lane	Volume Adjustment an	d Site			stics			/D				ND				20	
Number of Lanes (N) 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		-				 			11	<u> </u>	Τ,	_	 	, 1		_	Т
Line Assignment	Ni walan afil anaa (NI)		-		U	+-	_		U	_			0			+	-
Non-		U	0			10	10							0		-	1.7
Conflicting Lanes						+				-	N						LI
Volume (V), vehr/h				ne		+					Non-				IN		
Heavy Veh. Adj. (f _{hty}), % 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3			1			+				_		1		==		1 T	
Pedestrians Crossing				10.0	_			-				1	-	-		-	
Critical and Follow-Up Headway Adjustment		3			3	3			3	3	3		3	3	3		3
EB	Pedestrians Crossing		0				(0				0				0	
Left Right Bypass Left <th< td=""><td>Critical and Follow-Up</td><td>Headv</td><td>way Ac</td><td>ljustn</td><td>nent</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></th<>	Critical and Follow-Up	Headv	way Ac	ljustn	nent												
Critical Headway (sec) 5.1929				E	В			WB				NB				SB	
Follow-Up Headway (sec) 3.1858 3.			Left	Rig	ht I	Bypass	Left	Right	1	_	Left		Bypass	Lef	t F	Right	Bypass
EB	Critical Headway (sec)		5.1929	5.19	29	5.1929	5.1929	5.1929	5.19	29	5.1929	5.1929	5.1929	5.192	29 5.	1929	5.1929
EB	Follow-Up Headway (sec)		3.185	3.18	358	3.1858	3.1858	3.1858	3.18	358	3.1858	3.1858	3.1858	3.185	58 3.	1858	3.1858
Left Right Bypass Left	Flow Computations																
Circulating Flow (V _c), pc/h 906 84 62 619 Exiting Flow (V _{ex}), pc/h 62 0 159 844 Entry Flow (V _{ex}), pc/h 62 0 159 844 Entry Flow (V _{ex}), pc/h 694 84 752 287 Entry Volume veh/h 674 82 730 279 Capacity and v/c Ratios EB WB NB SB Left Right Bypass Lane Los B A B Lane Los F B A B B Lane 95% Queue 5.3 0.3 2.5 A Approach Los, s/veh B A B Intersection Delay, s/veh 7.63				E	В			WB				NB				SB	,
Exiting Flow (V _{ex}), pc/h Entry Flow (V _{ex}), pc/h Entry Flow (V _{ex}), pc/h Entry Volume veh/h Capacity and v/c Ratios EB			Left	Rig	tht l	Bypass	Left	Right	Вур	ass	Left	Right	Bypass	Lef	t F	Right	Bypass
Entry Flow (Ve), pc/h Entry Volume veh/h Capacity and v/c Ratios EB	Circulating Flow (V _c), pc/h			90	6			84				62				619	
Entry Volume veh/h	Exiting Flow (V _{ex}), pc/h			62	2			0				159				844	
EB	Entry Flow (V _e), pc/h							694				84	752			287	
EB	Entry Volume veh/h							674				82	730			279	
Left Right Bypass Left Right Right Bypas	Capacity and v/c Ratio	s															
Capacity (c _{PCE}), pc/h 0 1039 1063 608 Capacity (c), veh/h 0 1009 1032 591 v/c Ratio (X) 0.67 0.08 0.47 Delay and Level of Service EB WB NB SB Left Right Bypass				Е	В			WB				NB				SB	
Capacity (c), veh/h 0 1009 1032 591 V/c Ratio (X) 0.67 0.08 0.47 Delay and Level of Service EB WB NB SB Left Right Bypass Left Right Bypass Left Right Bypass Lane Control Delay (d), s/veh 13.8 4.2 0.0 13.8 Left Right Bypass			Left	Rig	ght I	Bypass	Left	Right	Вур	ass	Left	Right	Bypass	Lef	t F	Right	Bypass
V/c Ratio (X) 0.67 0.08 0.47 Delay and Level of Service Lane Control Delay (d), s/veh EB WB NB SB Left Right Bypass	Capacity (c _{PCE}), pc/h			0				1039				1063				608	
Delay and Level of Service EB WB NB SB Left Right Bypass Left Right Bypass Left Right Bypass Lane Control Delay (d), s/veh 13.8 4.2 0.0 13.8 Lane LOS F B A B Lane 95% Queue 5.3 0.3 2.5 Approach Delay, s/veh 13.76 0.42 13.76 Approach LOS, s/veh B A B Intersection Delay, s/veh 7.63 7.63	Capacity (c), veh/h			0				1009				1032				591	
EB	v/c Ratio (X)							0.67				0.08			(0.47	
Left Right Bypass Lane Control Delay (d), s/veh 13.8 4.2 0.0 13.8 Lane LOS F B A B Lane 95% Queue 5.3 0.3 2.5 Approach Delay, s/veh 13.76 0.42 13.76 Approach LOS, s/veh B A B Intersection Delay, s/veh 7.63 7.63	Delay and Level of Ser	vice															
Lane Control Delay (d), s/veh 13.8 4.2 0.0 13.8 Lane LOS F B A B Lane 95% Queue 5.3 0.3 2.5 Approach Delay, s/veh 13.76 0.42 13.76 Approach LOS, s/veh B A B Intersection Delay, s/veh 7.63			T	Е	В			WB				NB				SB	
Lane Control Delay (d), s/veh 13.8 4.2 0.0 13.8 Lane LOS F B A B Lane 95% Queue 5.3 0.3 2.5 Approach Delay, s/veh 13.76 0.42 13.76 Approach LOS, s/veh B A B Intersection Delay, s/veh 7.63			Left	Rig	ght	Bypass	Left	Right	Вур	ass	Left	Right	Bypass	Lef	t F	Right	Bypass
Lane LOS F B A B Lane 95% Queue 5.3 0.3 2.5 Approach Delay, s/veh 13.76 0.42 13.76 Approach LOS, s/veh B A B Intersection Delay, s/veh 7.63 7.63	Lane Control Delay (d), s/v	eh			1			13.8				4.2	0.0			13.8	
Approach Delay, s/veh 13.76 0.42 13.76 Approach LOS, s/veh B A B Intersection Delay, s/veh 7.63 7.63				F	:			В				Α			\top	В	
Approach Delay, s/veh 13.76 0.42 13.76 Approach LOS, s/veh B A B Intersection Delay, s/veh 7.63	Lane 95% Queue				\dashv			5.3				0.3			\top	2.5	
Approach LOS, s/veh B A B Intersection Delay, s/veh 7.63								13.76				0.42			1	3.76	
Intersection Delay, s/veh 7.63																В	
							1			7.	63			1			
											4					*	

					ROU	NDABO	UT REP	ORT								
General Information							Site In	form	atio	n						
Analyst DSK Agency or Co. Kelly E Date Performed 2/21/2	Enginee 022 eak Hou						Intersect E/W Str N/S Stre Analysis Project	ction eet N eet Na s Yea	lame ame	Pacii W 4ti Pacii 2025	ic Hwy.		et			
Project Description:																
Volume Adjustment an	d Site	Chara	acteris	stics												
		Е	В			V	/B				NB				SB	
	L	Т	R	U	L	Т	R	U	L	. T	R	U	L	Т	R	U
Number of Lanes (N)	0	0	0		0	0	0		0	1	0		0	1	0	
Lane Assignment							LR	?				Г				LT
Right-Turn Bypass		No	ne			No	ne		_	Non-	Yielding				None	
Conflicting Lanes		1	1				1				1				1	
Volume (V), veh/h				0	546		65	0		71		0	53	190	_	0
Heavy Veh. Adj. (f _{HV}), %	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3
Pedestrians Crossing		()			(0				0				0	
Critical and Follow-Up	Headv	vay A	djustr	nent												
			Е	В			WB				NB				SB	
		Left	Rig	ght	Bypass	Left	Right	Вур	ass	Left	Right	Bypass	Let	ft	Right	Bypass
Critical Headway (sec)		5.192	9 5.19	929	5.1929	5.1929	5.1929	5.19	929	5.1929	5.1929	5.1929	5.19	29	5.1929	5.1929
Follow-Up Headway (sec)		3.185	8 3.18	358	3.1858	3.1858	3.1858	3.18	858	3.1858	3.1858	3.1858	3.18	58	3.1858	3.1858
Flow Computations																
			Е	В			WB				NB				SB	
		Left	Ri	ght	Bypass	Left	Right	Вур	ass	Left	Right	Bypass	Let	ft	Right	Bypass
Circulating Flow (V _c), pc/h			93	34			84				63				646	
Exiting Flow (V _{ex}), pc/h			6	3			0				161	,			871	
Entry Flow (V _e), pc/h							723				84	760			288	
Entry Volume veh/h							702				82	738			280	
Capacity and v/c Ratio	s															
			Е	В			WB				NB				SB	
		Left	Ri	ght	Bypass	Left	Right	Вур	ass	Left	Right	Bypass	Le	ft	Right	Bypass
Capacity (c _{PCE}), pc/h			(1039				1061				592	
Capacity (c), veh/h			()			1009				1030				575	
v/c Ratio (X)							0.70				0.08				0.49	
Delay and Level of Ser	vice															
			E	В			WB				NB				SB	
		Left	Ri	ght	Bypass	Left	Right	Вур	oass	Left	Right	Bypass	Le	ft	Right	Bypass
Lane Control Delay (d), s/v	reh						14.8				4.2	0.0			14.5	
Lane LOS			F				В				Α				В	
Lane 95% Queue							5.9				0.3				2.6	
Approach Delay, s/veh							14.77				0.42				14.47	
Approach LOS, s/veh							В				Α				В	
Intersection Delay, s/veh									8.	19						
Intersection LOS									P	4						

						,,,,,,	UT REP									
General Information							Site In	form	atio	n						
Analyst DSK Agency or Co. Kelly E Date Performed 2/21/20 Time Period PM Pe Peak Hour Factor 0.94	Enginee 022 eak Hou						Intersect E/W Str N/S Stre Analysis Project	tion eet N eet Na Yea	ame	Paci W 4t	fic Hwy.	& 4th St.				
Project Description:	d C:40	Chara	oto viot	ioo												
Volume Adjustment an	a Site	Cnara		ics		W	/D		Γ		NB				SB	
	L	Т	R	U	L	T	R		L	_ T	T R	T u	L	Т	R	Tu
Number of Lanes (N)	0	0	0	+	0	0	0		0		0	-	0	1	0	+
Lane Assignment				-			LR		 			$\frac{1}{T}$			_	LT
Right-Turn Bypass		Nor)e			No				Non	l -Yielding		,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	N	_L lone	
Conflicting Lanes		1		_		110					1	+			1	
Volume (V), veh/h		<u> </u>		0	327		31	0		169		0	16	129		0
Heavy Veh. Adj. (f _{HV}), %	3	3	3	3	3	3	3	3	3		3	3	3	3	3	3
Pedestrians Crossing		0		-		()				0				0	
Critical and Follow-Up	Heady	vav An	iustme	ent												
Critical and I onow-op	ricadi	lay Au	EB				WB				NB		Π		SB	
		Left	Righ	t Byp	ass L	.eft	Right	Вур	ass	Left	Right	Bypass	Let	ft	Right	Bypass
Critical Headway (sec)		5.1929	+	_		1929	5.1929	_		5.1929	5.1929		5.19	-		5.1929
Follow-Up Headway (sec)		3.1858	+				3.1858	+		3.1858	3.1858	3.1858	3.18	_	.1858	3.1858
Flow Computations																
•			EB				WB				NB		T		SB	
		Left	Righ	t Byp	ass L	_eft	Right	Вур	ass	Left	Right	Bypass	Let	ft	Right	Bypass
Circulating Flow (V _c), pc/h			517				185				18				358	
Exiting Flow (V _{ex}), pc/h			18				0				219				500	
Entry Flow (V _e), pc/h	,						392				185	640			159	
Entry Volume veh/h							381				180	621			154	
Capacity and v/c Ratios	s															
			EB				WB				NB				SB	
		Left	Righ	t Byp	ass L	_eft	Right	Вур	ass	Left	Right	Bypass	Le	ft	Right	Bypass
Capacity (c _{PCE}), pc/h			0				939				1110				790	
Capacity (c), veh/h			0				912				1078				767	
v/c Ratio (X)							0.42				0.17				0.20	
Delay and Level of Ser	vice															
			EB				WB				NB				SB	
		Left	Righ	t Byp	ass L	_eft	Right	Вур	ass	Left	Right	Bypass	Le	ft	Right	Bypass
Lane Control Delay (d), s/ve	eh						8.8				4.8	0.0			6.9	
Lane LOS			F				Α				Α				Α	
Lane 95% Queue							2.1				0.6				0.7	
Approach Delay, s/veh							8.83				1.09				6.88	
Approach LOS, s/veh							Α				Α				Α	
Intersection Delay, s/veh									3.	96						
Intersection LOS									A	4						

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					ROU	NDABO	UT REP	ORT								
General Information							Site Int	orm:	atio	n						
Analyst DSK Agency or Co. Kelly E Date Performed 2/21/20 Time Period PM Pe Peak Hour Factor 0.94	-						Intersect E/W Str N/S Stre Analysis Project	tion eet Na eet Na Year	ame ame	Pacit W 4ti Pacit 2025	ic Hwy.	& 4th St.	t			
Project Description:	d Cita	Char	otori	otico												
Volume Adjustment an	a Site	Cnara		Sucs	1	١٨	/B		ı		NB			9	SB	
	L	Т	В R	U	+_	ТТ	R	U	L		R	U	L	T	R	U
Number of Lanes (N)	0	0	0		0	0	0		0	_	0		0	1	0	+
Lane Assignment					+	1 0	LR		<u> </u>		1 7	-				LT
Right-Turn Bypass		No.	ne		_	No.	ne			Non-	Yielding			N	one	
Conflicting Lanes		1					1				1				1	
Volume (V), veh/h				0	362		38	0		179		0	26	137		0
Heavy Veh. Adj. (f _{HV}), %	3	3	3	3	3	3	3	3	3	_	3	3	3	3	3	3
Pedestrians Crossing		ا ت			+		0				0				0	
Critical and Follow-Up	Hoady			nent												
Chucai and Follow-op	Heauv	Vay A	ijusti E				WB				NB				SB	
		Left	_		Bypass	Left	Right	Вур	ass	Left	Right	Bypass	Lef	t F	Right	Bypass
Critical Headway (sec)		5.192	_	-	5.1929		5.1929	_	_	5.1929	5.1929		5.192	_		5.1929
Follow-Up Headway (sec)			8 3.18				3.1858	+	-	3.1858	3.1858	3.1858	3.185	58 3.	1858	3.1858
Flow Computations								1								
•		Π	E	В			WB				NB				SB	
		Left	Rig	ght	Bypass	Left	Right	Вур	ass	Left	Right	Bypass	Lef	t I	Right	Bypass
Circulating Flow (V _c), pc/h			57	75			196				28				397	
Exiting Flow (V _{ex}), pc/h			2	8			0				238				547	
Entry Flow (V _e), pc/h				T			438				196	705			179	
Entry Volume veh/h							425				190	684			174	
Capacity and v/c Ratio	s															
		T	Е	В			WB				NB				SB	
		Left	Ri	ght	Bypass	Left	Right	Вур	ass	Left	Right	Bypass	Lef	t I	Right	Bypass
Capacity (c _{PCE}), pc/h			()			929				1098				760	
Capacity (c), veh/h			()			902				1066				738	
v/c Ratio (X)							0.47				0.18				0.24	
Delay and Level of Ser	vice															
			E	В			WB				NB				SB	
		Left	Ri	ght	Bypass	Left	Right	Вур	ass	Left	Right	Bypass	Lef	t	Right	Bypass
Lane Control Delay (d), s/v	eh						9.9				5.0	0.0		\perp	7.6	
Lane LOS			1	=			A				Α				Α	
Lane 95% Queue							2.6				0.6				0.9	
Approach Delay, s/veh							9.86				1.09		-		7.55	
Approach LOS, s/veh							Α				Α				Α	
Intersection Delay, s/veh										38						
Intersection LOS								4 oTM		4		Conor				2:40 DN

				ROU	NDABO	UT REP	ORT								
General Information					T	Site Inf	orma	tion	7						
Analyst DSK Agency or Co. Kelly E Date Performed 2/21/20	nginee 022 ak Hou					Intersect E/W Stre N/S Stre Analysis Project I	tion eet Na eet Nar Year	me	Pacif W 4tl Pacif 2025	ic Hwy.		et			
Project Description:															
Volume Adjustment an	d Site	Charac	teristic	S											
		EB			W	/B				NB			SB		
	L	Т	Rι	J L	Т	R	U	L	Т	R	U	L	Т	R	U
Number of Lanes (N)	0	0	0	0	0	0		0	1	0		0	1	0	
Lane Assignment						LR				1					LT
Right-Turn Bypass		Non	Э		No	ne			Non-	Yielding			Non	е	
Conflicting Lanes		1			,	1				1			1		
Volume (V), veh/h			(380		39	0		179		0	28 1	37		0
Heavy Veh. Adj. (f _{HV}), %	3	3	3 3	3	3	3	3	3	3	3	3	3	3	3	3
Pedestrians Crossing		0			()				0			0		
Critical and Follow-Up	Headv	vay Adj	iustmen	it											
-			EB			WB				NB			SE	3	
		Left	Right	Bypass	Left	Right	Вура	iss	Left	Right	Bypass	Left	Rig	ht	Bypass
Critical Headway (sec)		5.1929	5.1929	5.1929	5.1929	5.1929	5.192	29 8	5.1929	5.1929	5.1929	5.1929	5.19	29	5.1929
Follow-Up Headway (sec)		3.1858	3.1858	3.1858	3.1858	3.1858	3.18	58 3	3.1858	3.1858	3.1858	3.1858	3.18	58	3.1858
Flow Computations															
			EB			WB				NB			SI	3	
		Left	Right	Bypass	Left	Right	Вура	ass	Left	Right	Bypass	Left	Rig	ht	Bypass
Circulating Flow (V _c), pc/h			597			196				31			41	6	
Exiting Flow (V _{ex}), pc/h			31			0				239			56	7	
Entry Flow (V _e), pc/h						459				196	735		18	1	
Entry Volume veh/h						446				190	714		17	6	
Capacity and v/c Ratio	s														
			EB			WB				NB			SI	В	
		Left	Right	Bypass	Left	Right	Вура	ass	Left	Right	Bypass	Left	Rig	ht	Bypass
Capacity (c _{PCE}), pc/h			0			929				1096			74	5	
Capacity (c), veh/h			0			902				1064			72	3	
v/c Ratio (X)						0.49				0.18			0.2	24	
Delay and Level of Ser	vice														
			EB			WB				NB			S	В	
		Left	Right	Bypass	Left	Right	Вура	ass	Left	Right	Bypass	Left	Rig	ght	Bypass
Lane Control Delay (d), s/v	eh					10.3				5.0	0.0		7.	8	
Lane LOS			F			В				Α			A	1	
Lane 95% Queue						2.8				0.6			1.	0	
Approach Delay, s/veh						10.29	•			1.05			7.7	79	
Approach LOS, s/veh						В				Α			A	١	
Intersection Delay, s/veh								4.5	53						
Intersection LOS							4	Α							
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	11	NO-WAY STOP	CONTRO	L SUM	WAR	K Y				
General Information			Site In	formati	ion					
Analyst Agency/Co. Date Performed Analysis Time Period	DSK Kelly Engi 2/21/2022 AM Peak		Interse Jurisdio Analysi	ction			La Ctr. Ro City of La 2022		men F	₹d.
Project Description Exis	sting					,				
East/West Street: La Cer	nter Road		North/S	outh Stre	eet:	Timmen	Rd.			
ntersection Orientation:	East-West		Study P	eriod (h	rs):	0.25				
Vehicle Volumes and	Adjustments									
Major Street		Eastbound					Westbou	ind		
Movement	1	2	3			4	5			3
	L	Т	R			L	T		F	₹
Volume (veh/h)		288	8			61	622			
Peak-Hour Factor, PHF	0.93	0.96	0.96		0.	.96	0.96		0.9	93
Hourly Flow Rate, HFR veh/h)	0	300	8			53	647		0)
Percent Heavy Vehicles	0					0				-
Median Type				Undivid	ded		,			
RT Channelized			0						C)
anes	0	1	0			1	1		C)
Configuration			TR			L	T			
Upstream Signal		0					0			
Minor Street		Northbound					Southbou	und		
Movement	7	8	9			10	11			2
	L	T	R			L	T			₹
Volume (veh/h)	11		. 46							
Peak-Hour Factor, PHF	0.96	1.00	0.96		0	.93	1.00		0.9	93
Hourly Flow Rate, HFR (veh/h)	11	0	47			0	0		(
Percent Heavy Vehicles	2	0	2			0	0)
Percent Grade (%)		0					0			
Flared Approach		N					N			
Storage		0					0			
RT Channelized			0							
Lanes	1	0	1			0	0		()
Configuration	L		R							
Delay, Queue Length, ar	nd Level of Serv	rice								
Approach	Eastbound	Westbound		Northbou	und			Southbo	und	
Movement	1	4	7	8		9	10	11		12
Lane Configuration		L	L			R				
v (veh/h)		63	11			47				
C (m) (veh/h)		1264	230			736				
v/c		0.05	0.05			0.06				
95% queue length		0.16	0.15			0.20				
Control Delay (s/veh)		8.0	21.4			10.2				
LOS		Α	С			В				
Approach Delay (s/veh)				12.4						
Approach LOS				В						

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Analyst DSK Agency/Co. Kelly Engineering Date Performed 2/21/2022 Analysis Time Period AM Peak Hour			Interse Jurisdio Analysi	ction	ion	La Ctr. Rd. & Timmen Rd. City of La Center 2025			
•									
Project Description Year 2025 w/o Project East/West Street: La Center Road			North/S	outh Str	eet: Timmen	Rd.			
ntersection Orientation:					rs): 0.25				
/ehicle Volumes and									
Wajor Street	lagaotinonto	Eastbound				Westbour	ıd		
Movement	1	2	3		4	5		6	
	L	Т	R		L	Т		R	
/olume (veh/h)		340	9		68	699		0.00	
Peak-Hour Factor, PHF	0.93	0.96	0.96		0.96	0.96		0.93	
Hourly Flow Rate, HFR veh/h)	0	354	9		70	728		0	
Percent Heavy Vehicles	0				0				
Median Type			Undivided						
RT Channelized			0				0		
anes	0	1	0 1		1		0		
Configuration			TR		L	T			
Jpstream Signal		0				0			
Minor Street		Northbound				Southbou	nd		
Movement	7	8	9		10	11		12	
	L	Т	R		L	T		R	
/olume (veh/h)	12		49		4.00		0.00		
Peak-Hour Factor, PHF	0.96	1.00	0.96		0.93	1.00		0.93	
Hourly Flow Rate, HFR veh/h)	12	0	51		0	0		0	
Percent Heavy Vehicles	2	0	2		0	0		0	
Percent Grade (%)		0		-		0			
-lared Approach		N				N			
Storage		0				0	_		
RT Channelized			0	-				0	
_anes	1	0	1		0	0		0	
Configuration	L		R						
Delay, Queue Length, a						1			
Approach	Eastbound	Westbound		Northbo	und	S	outhbound		
Movement	1	4	7	8	9	10	11	12	
Lane Configuration		L	L		R				
v (veh/h)		70	12		51				
C (m) (veh/h)		1207	186		686				
v/c		0.06	0.06		0.07				
95% queue length		0.18	0.20		0.24				
Control Delay (s/veh)		8.2	25.7		10.7				
LOS		Α	D		В				
Approach Delay (s/veh)				13.5					
(arveil)						-			

0 11.6 (Cita In	t	ion							
Analyst DSK Agency/Co. Kelly Engineering Date Performed 2/21/2022 Analysis Time Period AM Peak Hour			Site In Interse Jurisdid Analysi	ction	ion	La Ctr. Rd. & Timmen Rd. City of La Center 2025						
Project Description Yea	ar 2025 with Proj	ect			· · · · · · · · · · · · · · · · · · ·							
East/West Street: La Center Road				North/South Street: Timmen Rd.								
Intersection Orientation:	East-West		Study P	eriod (h	nrs): 0.25	5						
Vehicle Volumes and	Adjustments											
Major Street		Eastbound		W				Westbound				
Movement	1	2	3		4		5		6			
	L	T	R		L		T		R			
Volume (veh/h)		347	9		71		719		0.02			
Peak-Hour Factor, PHF	0.93	0.96	0.96		0.96		0.96		0.93			
Hourly Flow Rate, HFR (veh/h)	0	361	9		73		748		0			
Percent Heavy Vehicles	0				0							
Median Type			Undivided									
RT Channelized			0					0				
Lanes	0	1	0		1		1		0			
Configuration			TR		L		T					
Upstream Signal		0					0					
Minor Street		Northbound	Southbo				Southbou	nd				
Movement	7	8	9		10		11		12			
	L	Т	R		L		T		R			
Volume (veh/h)	12		49									
Peak-Hour Factor, PHF	0.96	1.00	0.96	0.96 0.93			1.00		0.93			
Hourly Flow Rate, HFR (veh/h)	12	0	51		0		0		0			
Percent Heavy Vehicles	2	0	2		0		0		0			
Percent Grade (%)		0					0					
Flared Approach		N					N					
Storage		0					0					
RT Channelized			0						0			
Lanes	1	0	1		0		0		0			
Configuration	L		R									
Delay, Queue Length, a	nd Level of Sen	vice										
Approach	Eastbound	Westbound		Northbo	und		S	Southbound	nbound			
Movement	1	4	7	8		9	10	11	12			
Lane Configuration		L	L		F	?						
v (veh/h)		73	12		5	1						
C (m) (veh/h)		1200	177		67							
v/c		0.06	0.07		0.0	08						
95% queue length		0.19	0.22		0.2				1			
Control Delay (s/veh)		8.2	26.8		10				+			
LOS		A A	D		E				1			
Approach Delay (s/veh)				13.8				1				
				B								
Approach LOS	orida, All Rights Rese		<u> </u>	Ъ			1	erated: 2/21/2				

2/21/2022

	Т	WO-WAY STOP	CONTRO	L SUN	IMARY							
General Information			Site In	format	ion							
Analyst DSK Agency/Co. Kelly Engineering Date Performed 2/21/2022 Analysis Time Period PM Peak Hour			Interse Jurisdi Analys			City	La Ctr. Rd. & Timmen Rd. City of La Center 2022					
	sting											
East/West Street: La Center Road				North/South Street: <i>Timmen Rd.</i> Study Period (hrs): 0.25								
Intersection Orientation:			Study F	eriod (h	irs): 0.25							
Vehicle Volumes and	Adjustments											
Major Street	Eastbound					Wes	Westbound					
Movement	1	2	3		4		<u>5</u> 		6			
) / L / L / L	L	639	R 20		L 67		1 421		R			
Volume (veh/h) Peak-Hour Factor, PHF	0.93	0.96	0.96		0.96		421 0.96		0.93			
Hourly Flow Rate, HFR (veh/h)	0.93	665	20		69		438		0.93			
Percent Heavy Vehicles	0				0							
Median Type				Undivi								
RT Channelized			0	T								
Lanes	0	1	0		1		1		0			
Configuration		'	TR		L	_	T					
Upstream Signal		0	111				0					
				-		Cou	Southbound					
Minor Street	7	Northbound 8	9		10		11		12			
Movement	/	T	R	-	L				R			
Volume (veh/h)	25		126				· '		11			
Peak-Hour Factor, PHF	0.96	1.00	0.96 0.93			1.00		0.93				
Hourly Flow Rate, HFR (veh/h)	26	0	131		0		0		0			
Percent Heavy Vehicles	8	0	2		0		0		0			
Percent Grade (%)		0	•				0					
Flared Approach		N					N					
Storage		0					0					
RT Channelized			0						0			
Lanes	1	0	1		0		0		0			
Configuration	L		R									
Delay, Queue Length, ar		vice										
Approach	Eastbound	Westbound		Northbo	und		S	outhboun	d			
Movement	1	4	7	8	9	1	0	11	12			
Lane Configuration	1	L	L	"	R				12			
v (veh/h)		69	26		131							
C (m) (veh/h)		918	171		454							
v/c		0.08	0.15		0.2							
95% queue length		0.24	0.52		1.1							
Control Delay (s/veh)		9.2	29.8		16.							
LOS		A	D		С							
Approach Delay (s/veh)				18.4								
Approach LOS				С								
		L										

	T\	NO-WAY STOP	CONTRO	LSUM	IMARY							
General Information			Site In	format	ion							
Analyst DSK Agency/Co. Kelly Engineering Date Performed 2/21/2022 Analysis Time Period PM Peak Hour			Intersed Jurisdid Analysi	ction			La Ctr. Rd. & Timmen Rd. City of La Center 2025					
Project Description Yea		ect										
East/West Street: La Center Road				North/South Street: Timmen Rd. Study Period (hrs): 0.25								
Intersection Orientation:			Study P	erioa (n	rs): 0.25							
Vehicle Volumes and	Adjustments					\\\/\						
Major Street	1	Eastbound	3		4	Westbou 5	ina	6				
Movement	 	2 	R		- L	T		R				
Volume (veh/h)	-	691	21		71	456						
Peak-Hour Factor, PHF	0.93	0.96	0.96		0.96	0.96		0.93				
Hourly Flow Rate, HFR (veh/h)	0	719	21		73	475		0				
Percent Heavy Vehicles	0				0							
Median Type				Undivi	ded							
RT Channelized			0					0				
Lanes	0	1	0		1	1		0				
Configuration			TR		L	T						
Upstream Signal		0				0						
Minor Street		Northbound S					und					
Movement	7	8	9		10	11		12				
	L	T	R		L	T		R				
Volume (veh/h)	25		133			1.00		0.00				
Peak-Hour Factor, PHF	0.96	1.00	0.96	-	0.93	1.00		0.93				
Hourly Flow Rate, HFR (veh/h)	26	0	138		0	0		0				
Percent Heavy Vehicles	8	0	2		0							
Percent Grade (%)		0	1			0						
Flared Approach		N				N O						
Storage		0				0		0				
RT Channelized			0		0	0		0				
Lanes	1	0	1 P	-	U	- 0		U				
Configuration	L		R									
Delay, Queue Length, a						T	0 - 41-1	-1				
Approach	Eastbound	Westbound		Northbo			Southbour					
Movement	1	4	7	8	9	10	11	12				
Lane Configuration		L	L		R							
v (veh/h)		73	26		138							
C (m) (veh/h)		876	148		422							
v/c		0.08	0.18		0.33							
95% queue length		0.27	0.61		1.40							
Control Delay (s/veh)		9.5	34.4		17.6							
LOS		Α	D		С							
Approach Delay (s/veh)				20.3								
Approach LOS				С								

General Information			Site In	format	tion						
Analyst DSK Agency/Co. Kelly Engineering Date Performed 2/21/2022 Analysis Time Period PM Peak Hour			Interse	Site Information Intersection Jurisdiction Analysis Year				La Ctr. Rd. & Timmen Rd. City of La Center 2025			
Project Description Year	r 2025 with Proj	ect									
East/West Street: La Center Road			North/South Street: Timmen Rd.								
Intersection Orientation:	East-West		Study P	eriod (ł	nrs): 0.25						
Vehicle Volumes and A	Adiustments										
Major Street		Eastbound				Westbour	nd				
Movement	1	2	3		4		5		6		
	L	Т	R		L		Т		R		
Volume (veh/h)		715	21		74		471				
Peak-Hour Factor, PHF	0.93	0.96	0.96		0.96		0.96		0.93		
Hourly Flow Rate, HFR (veh/h)	0	744	21		77		490		0		
Percent Heavy Vehicles	0				0						
Median Type			Undivided								
RT Channelized			0		(0				
Lanes	0	1	0		1		1		0		
Configuration			TR		L		T				
Upstream Signal		0					0				
Minor Street	1	Northbound					Southbound				
Movement	7	8	9	9			11		12		
	L	Т	R		L		Т		R		
Volume (veh/h)	26		133								
Peak-Hour Factor, PHF	0.96	1.00	0.96		0.93		1.00		0.93		
Hourly Flow Rate, HFR (veh/h)	27	0	138		0		0		0		
Percent Heavy Vehicles	8	0	2		0		0	0			
Percent Grade (%)		0					0				
Flared Approach		N					N				
Storage		0					0				
RT Channelized			0						0		
Lanes	1	0	1		0		0		0		
Configuration	L		R				8				
Delay, Queue Length, an		vice									
Approach	Eastbound	Westbound		Northbo	ound		Southb		bound		
Movement	1	4	7	8	94114)	10	11	12		
Lane Configuration		L	L		R						
v (veh/h)		77	27		13	8					
C (m) (veh/h)		857	137		40	9					
v/c		0.09	0.20		0.3	4					
95% queue length		0.30	0.70		1.4	6					
Control Delay (s/veh)		9.6	37.6		18.	2					
LOS		Α	· E		C						
Approach Delay (s/veh)				21.4	4						
Approach LOS			С								

APPENDIX F REFERENCES

References

- 1. <u>Trip Generation Manual</u>, 10th Edition, 2017, Institute of Transportation Engineers.
- 2. <u>Highway Capacity Manual</u>, 2000 and 2010, Transportation Research Board, National Research Council.
- 3. Correspondence with staff from the City of La Center.
- 4. Traffic Analysis Report, Lockwood Meadows Subdivision, Charbonneau Engineering, August 2021.